

# Metal Structures II

## Lecture V

### Long span structures

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# Introduction

Photo: syrek.com.pl



Photo: tatasteelurope.com

Distance between supports for office buildings is about 10 m.

Distance between supports for industry halls reaches 40-50.



Photo: metroland.com.au



Photo: ocmer.com.p



Photo: traskostal.pl

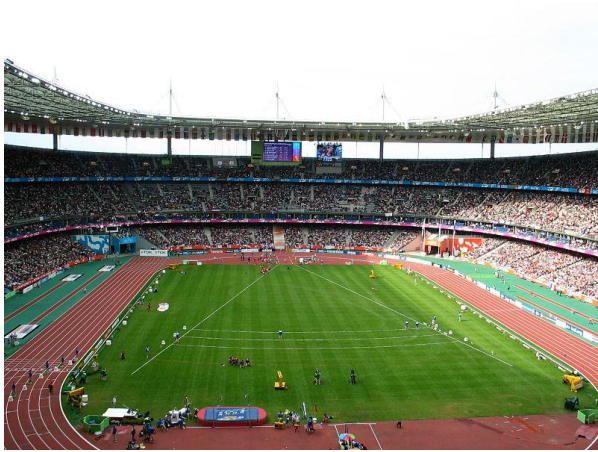


Photo: wikipedia

Photo: holland.com



But sometimes we need more than 100 meters between supports for roofs...



Photo: wikipedia

...and more than 1000 m for other type of structures.

Long span structures in the most often cases are intended for:

- Exhibition hall & exhibition centres
- Sport facilities
- Amusement centres
- Railway stations & airport halls
- Special industrial facilities
- Special transport facilities
- Landmarks
- Bridges



Photo: e-architect.co.uk, ballparkofbaseball.com, wikipedia, glebokie4.pl, rodaxx.com

# Steel hall

Hall: repetitive arrangement of flat frames (steel or concrete), interrelated by bracings.



Photo: shaymurtagh.co.uk

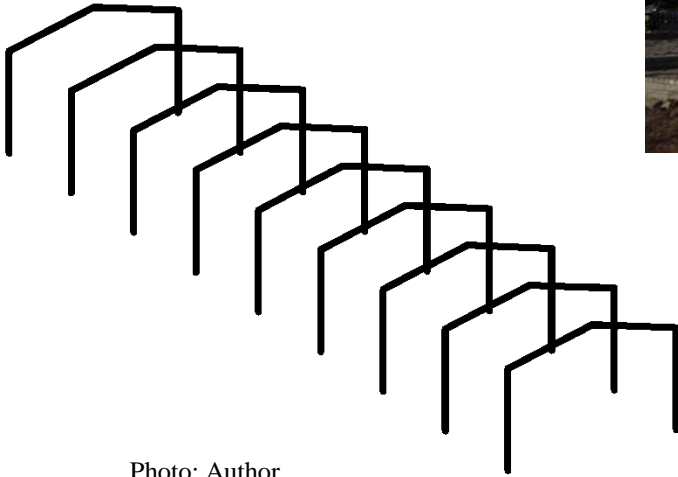


Photo: Author



Photo: weldon.pl

# Halls: industrial, warehouse, trade, cooler, garage, hangar, office, agricultural, sport, exhibition...

Photo: steel.com.au



Photo: easyhalls.com



Photo: ekbud.lublin.pl



Photo: toiowo.eu

Photo: pebsteel.com



Photo: aviationbuildingsystem.com



Photo: internationalsteelspan.com



Photo: sztuka-architektury.pl

Based on types of loads, there are two possibilities for steel hall:

- "heavy" hall with cranes act on hall structure;
- "light" hall without crane act on hall structure;

Photo: eci.com.pl



Photo: spawstal.pl



Photo: weldon.pl

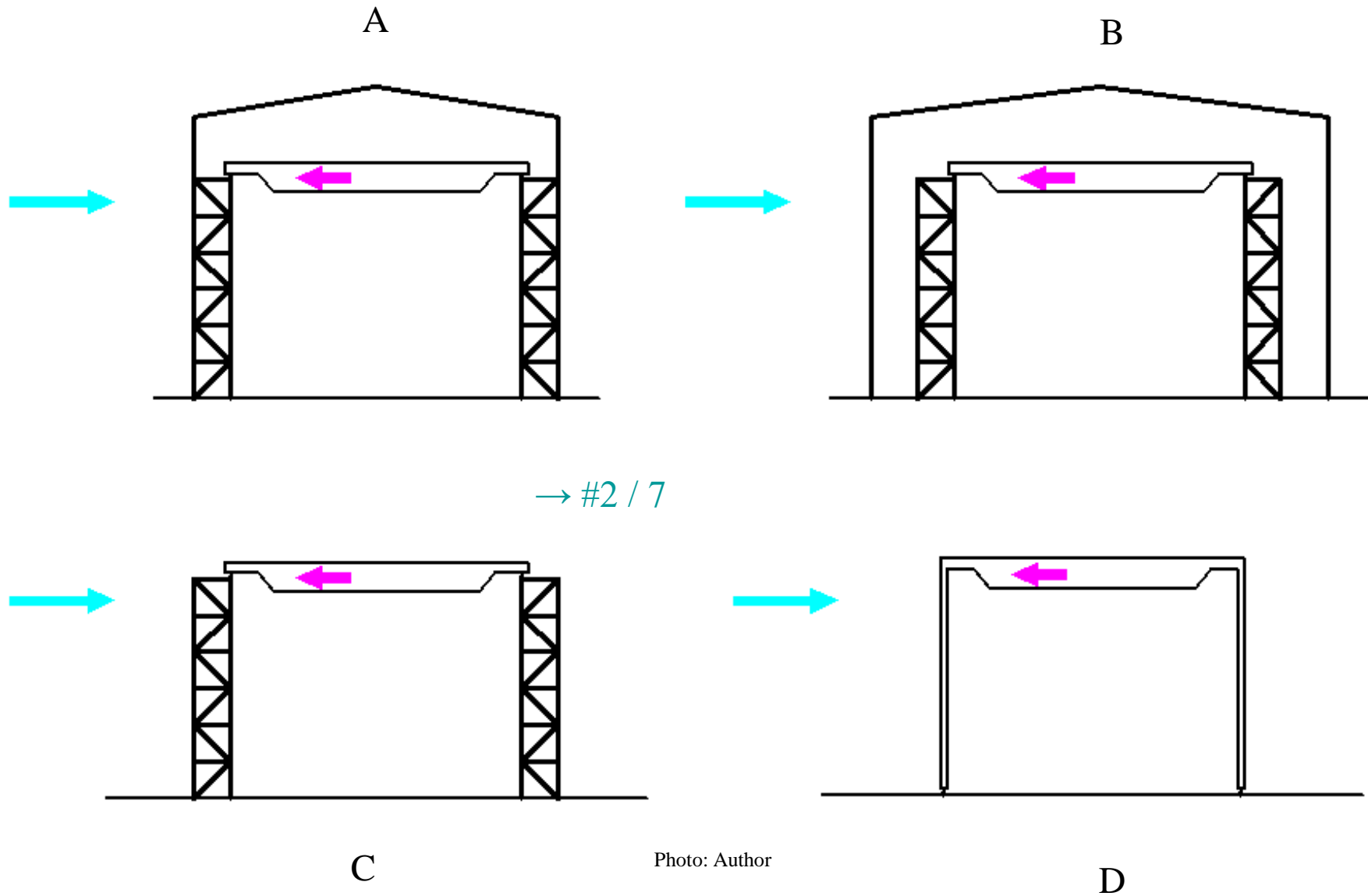


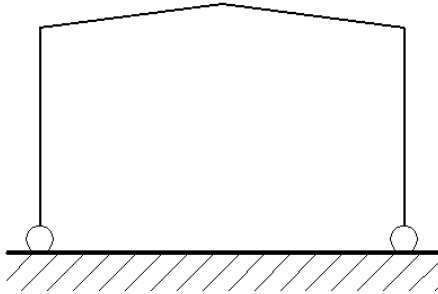
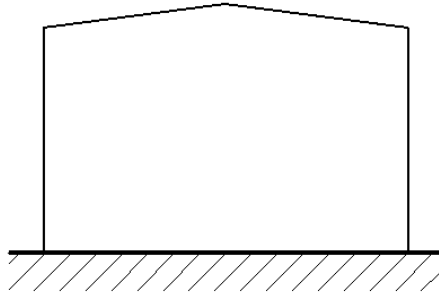
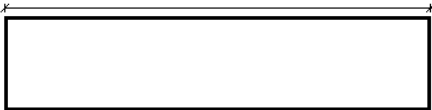
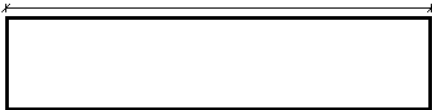
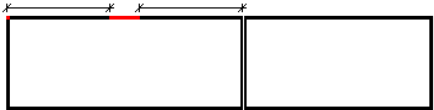
Photo: Author

According to situation A-D, there are three possibilities:

Situation	Type of hall
A	"Heavy"
B	"Light"
C	No hall
D	
Hall without crane	"Light"

# General rules

Photo: Author

	"Light" hall	"Heavy" hall
Static scheme		
Max length of building / max distance between dilatations	150 m 	120 m 
Max longitudinal distance between front wall / dilatation and vertical wall bracing	60 m 	

## Value of loads



Photo: Author

Width of hall 30,0 m

Longitudinal distance between columns 12,0 m

Vertical loads act on 4 columns on area 30,0 x 12,0 (360,0 m<sup>2</sup>):

	"Light" hall	"Heavy" hall
Snow (~ 1,50 kN / m <sup>2</sup> )	540 kN	540 kN
Wind (~ 0,60 kN / m <sup>2</sup> )	216 kN	216 kN
Roofing (~ 0,15 kN / m <sup>2</sup> )	54 kN	54 kN
16x purlins (~ 0,25 kN / m)	230 kN	230 kN
2x roof girders (~ 3,25 kN / m)	195 kN	195 kN
Crane (30 m span) + hoist load		520 – 2035 kN
<b>SUM</b>	<b>1235 kN</b>	<b>1755 – 3270 kN</b>
Proportion	1,0	1,42 – 2,65

Each structure can be presented as a complex of supermembers. External part for each structure is roofing/housing. For steel hall supermembers can be presented as follow:

- roofing and housing
- purlins
- wall girts
- bracings
- truss
- beams, girders, columns
- joints

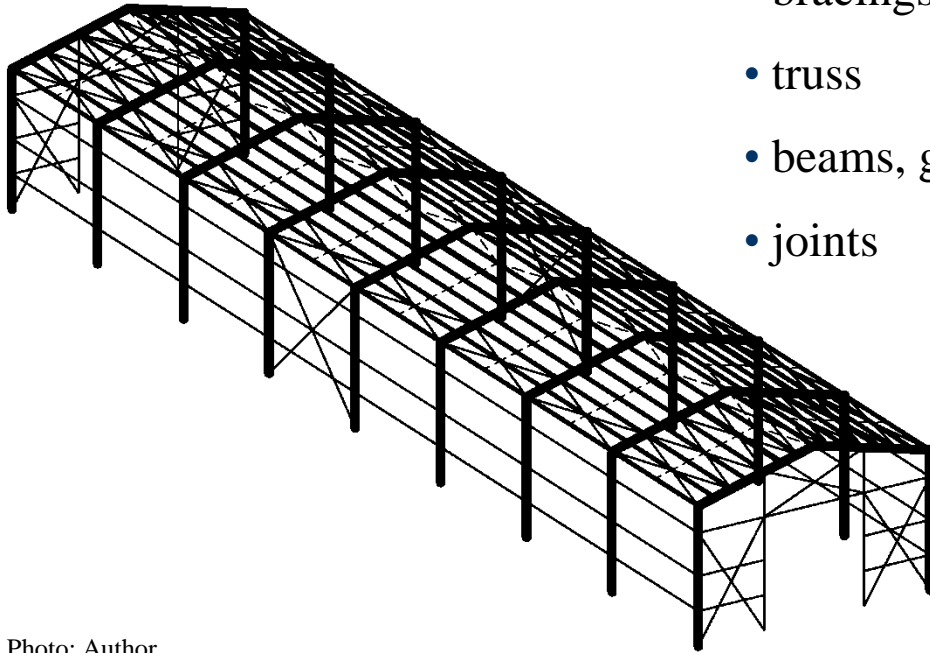
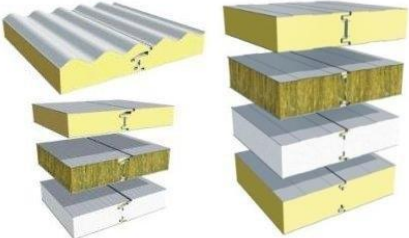



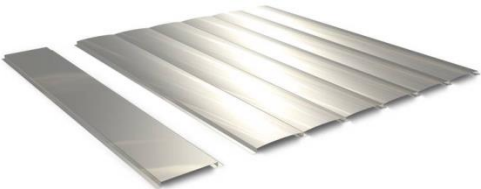









Photo: Author

Roofing / housing	Thermal isolation	Factory-made connecting latch	Anti-buckling protection for purlins according to EN
 <p>Photo: steelprofil.pl</p>			
 <p>Photo: pruszynski.com.pl</p>			
 <p>Photo: amarodachy.pl</p>			 (per 5 - 10 years from erection)

# Skylights

(not always)



Photo: globalprayers.info



















Photo: euroexport.pl



Photo: euroexport.pl

System solutions

## Recommended types of purlins for different length of span (distance between supports)

Length	Continous, cold-formed	Continous, suspended cold formed	One-span hot-rolled	Castellated	Truss
< 3					
3 – 4					
4 – 6					
6 – 8					
8 – 9					
9 – 12					
12 – 18					

# Roof bracings, most often used types

## "Heavy" hall - roof girders

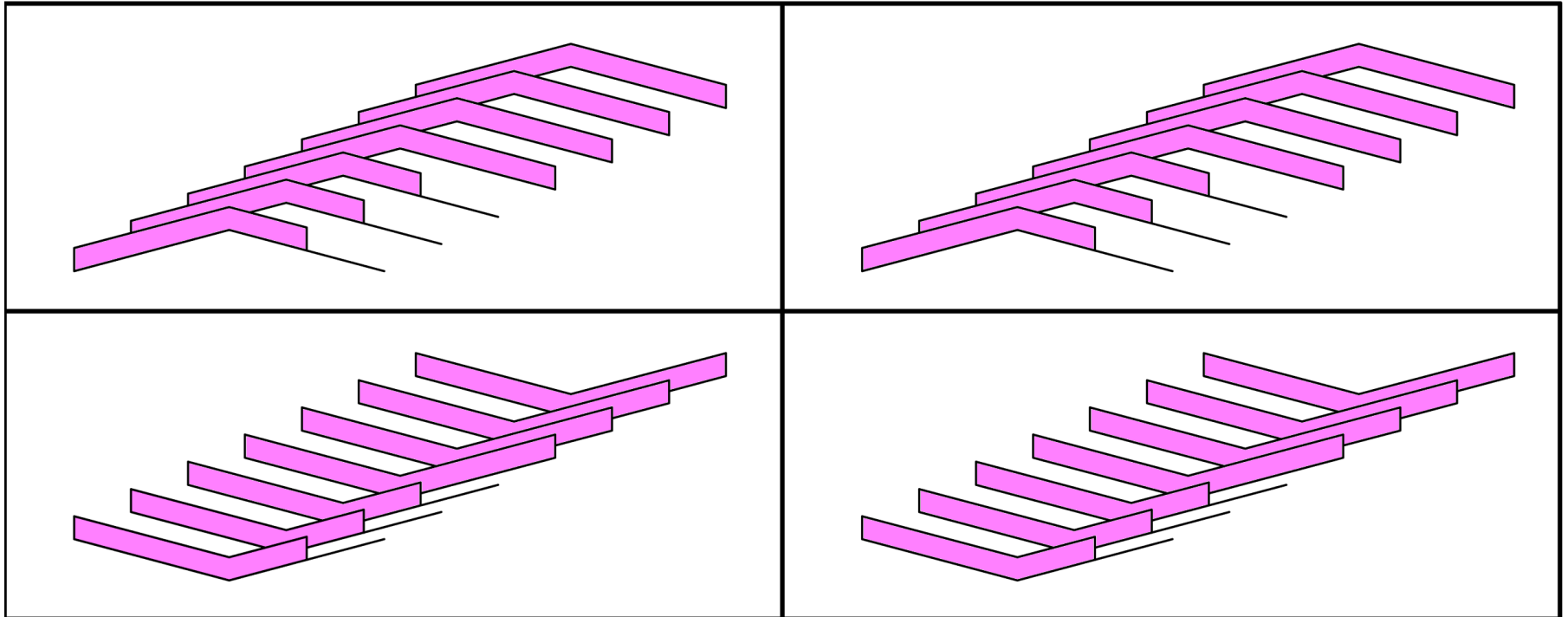
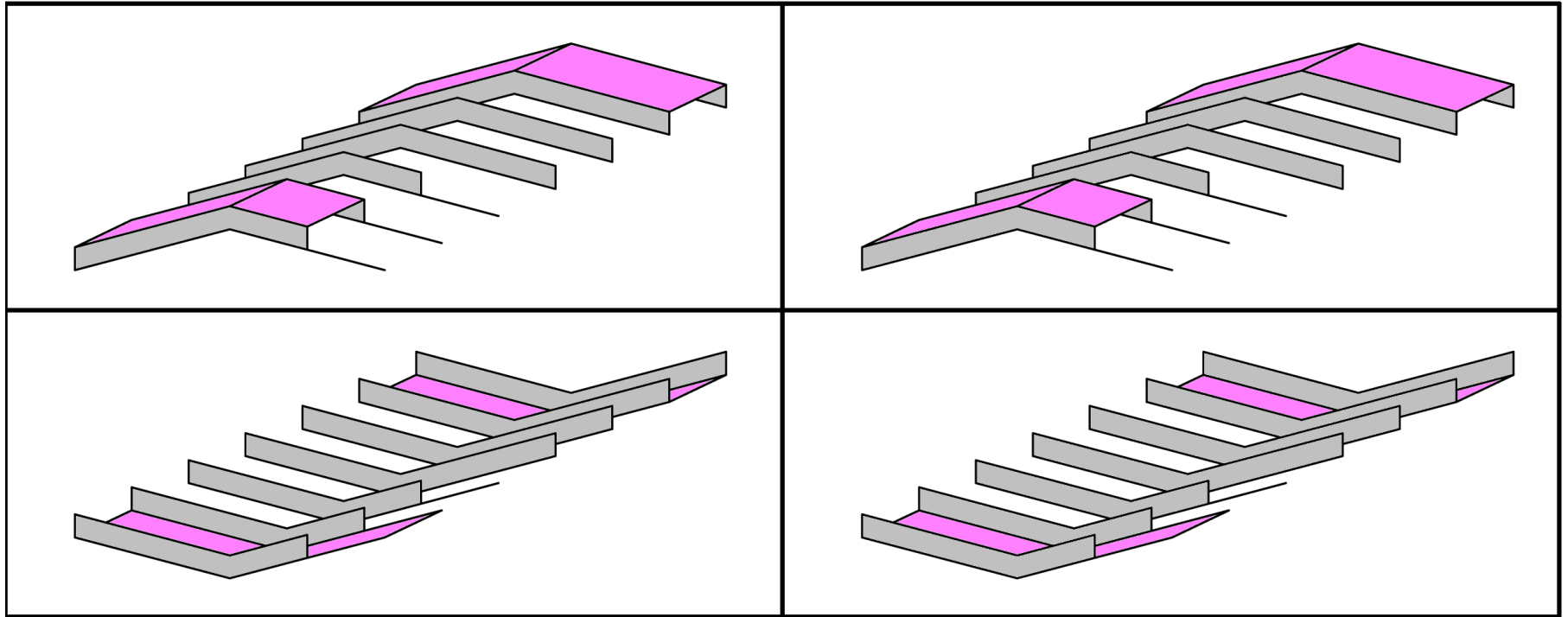


Photo: Author

## "Light" hall - roof girders

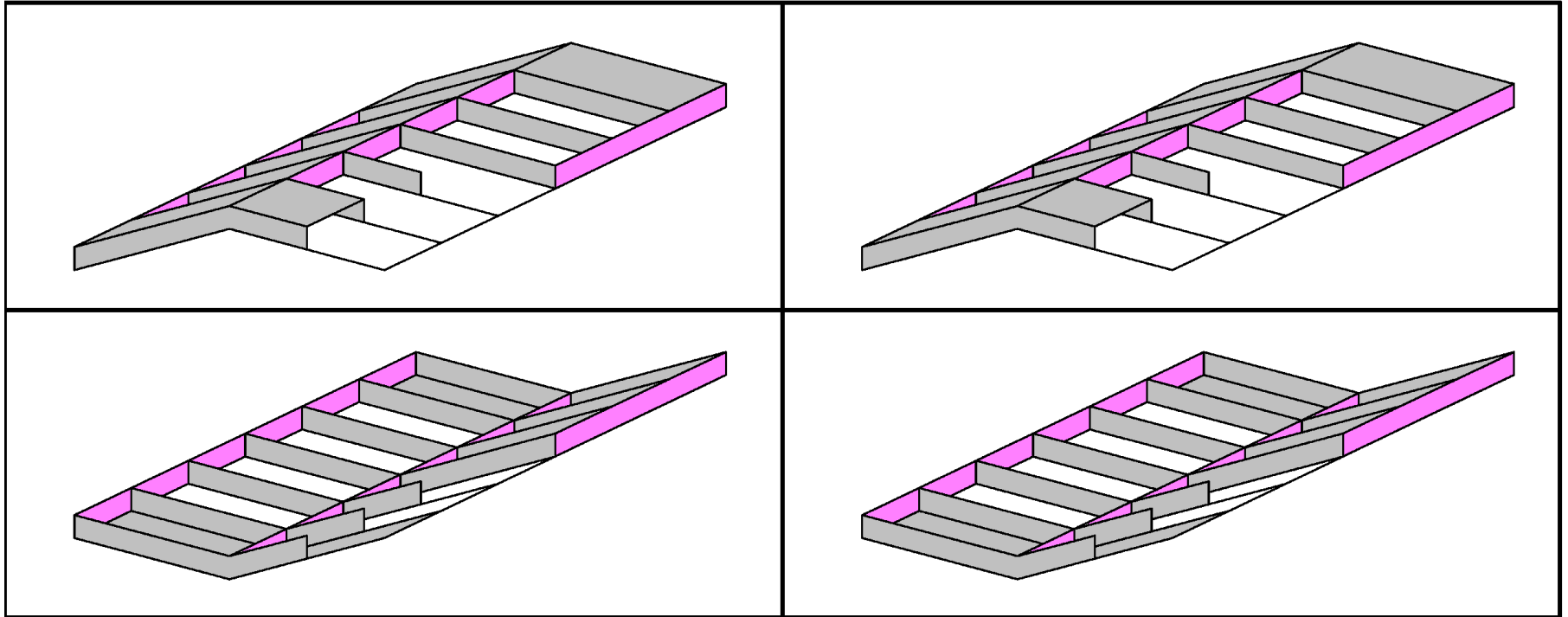
"Heavy" hall - horizontal upper transversal bracings



"Light" hall - horizontal upper transversal bracings

Photo: Author

## "Heavy" hall - vertical longitudinal bracings



## "Light" hall - vertical longitudinal bracings

Photo: Author

## "Heavy" hall - horizontal upper longitudinal bracings

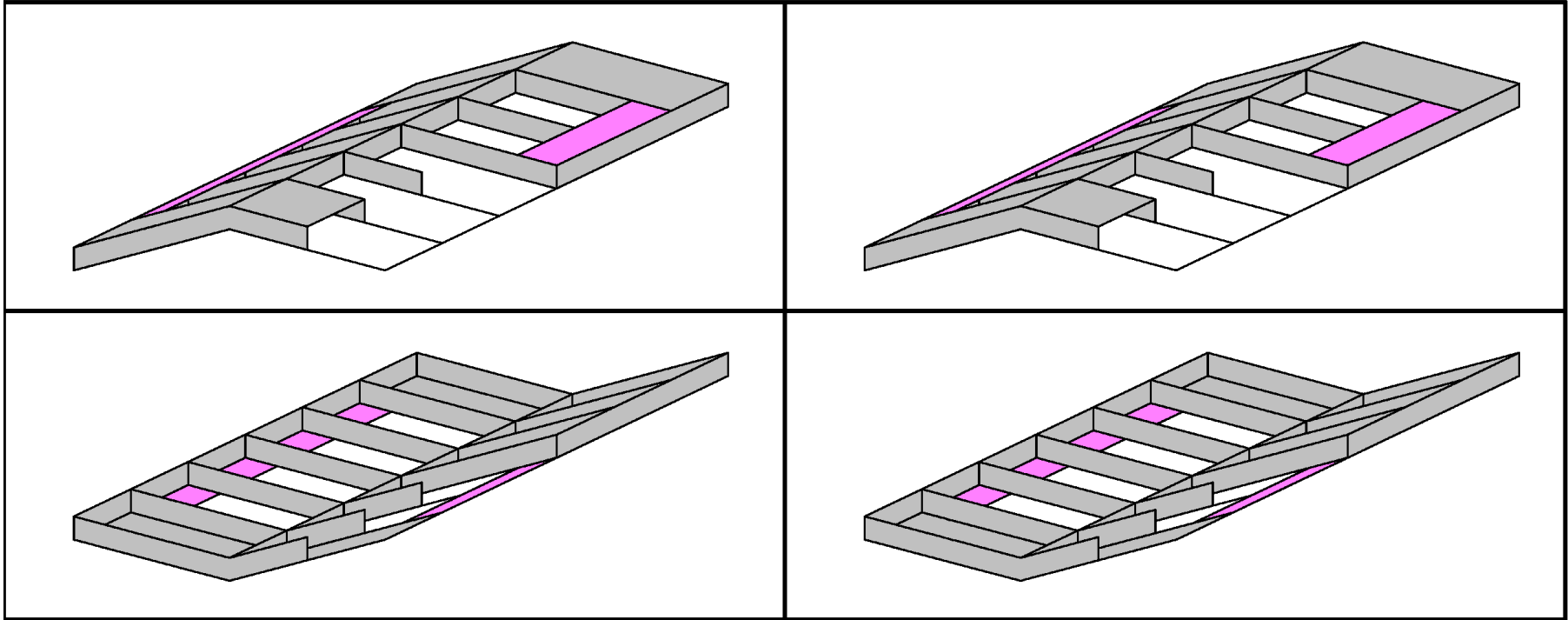
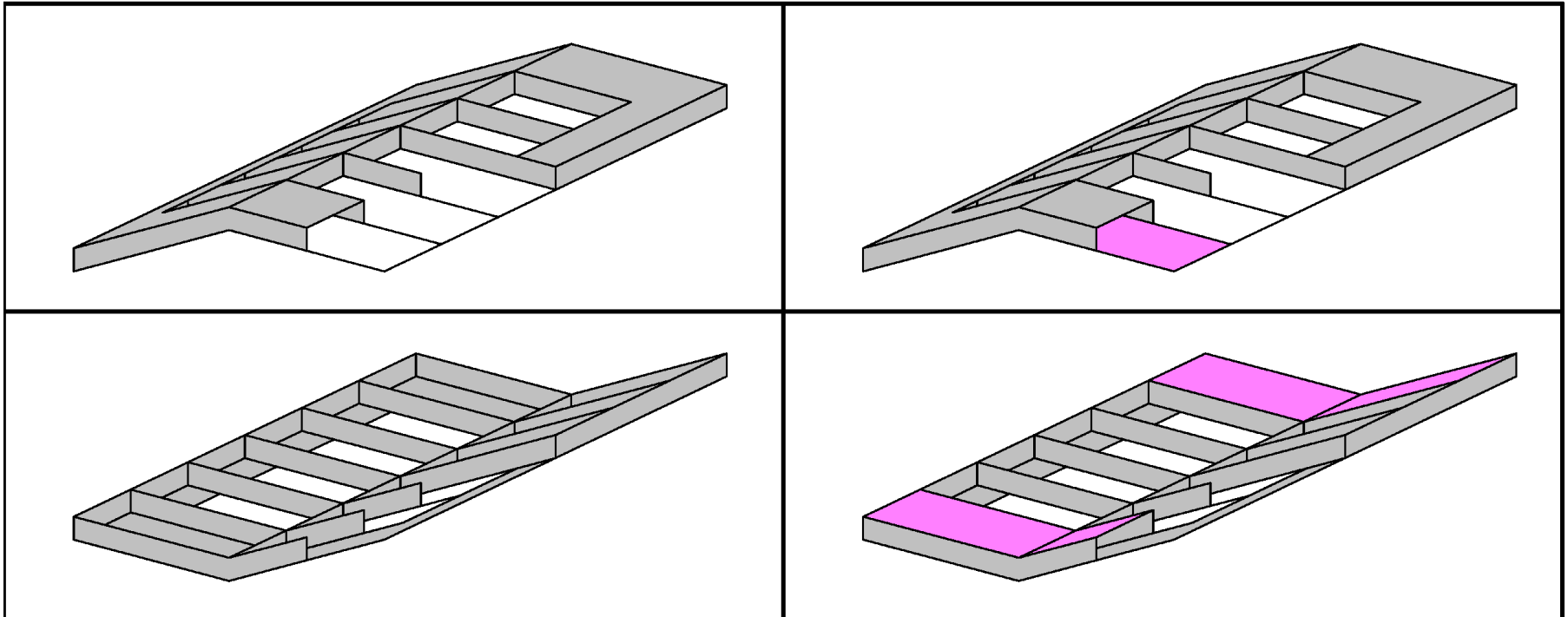


Photo: Author

## "Light" hall - horizontal upper longitudinal bracings (not always)

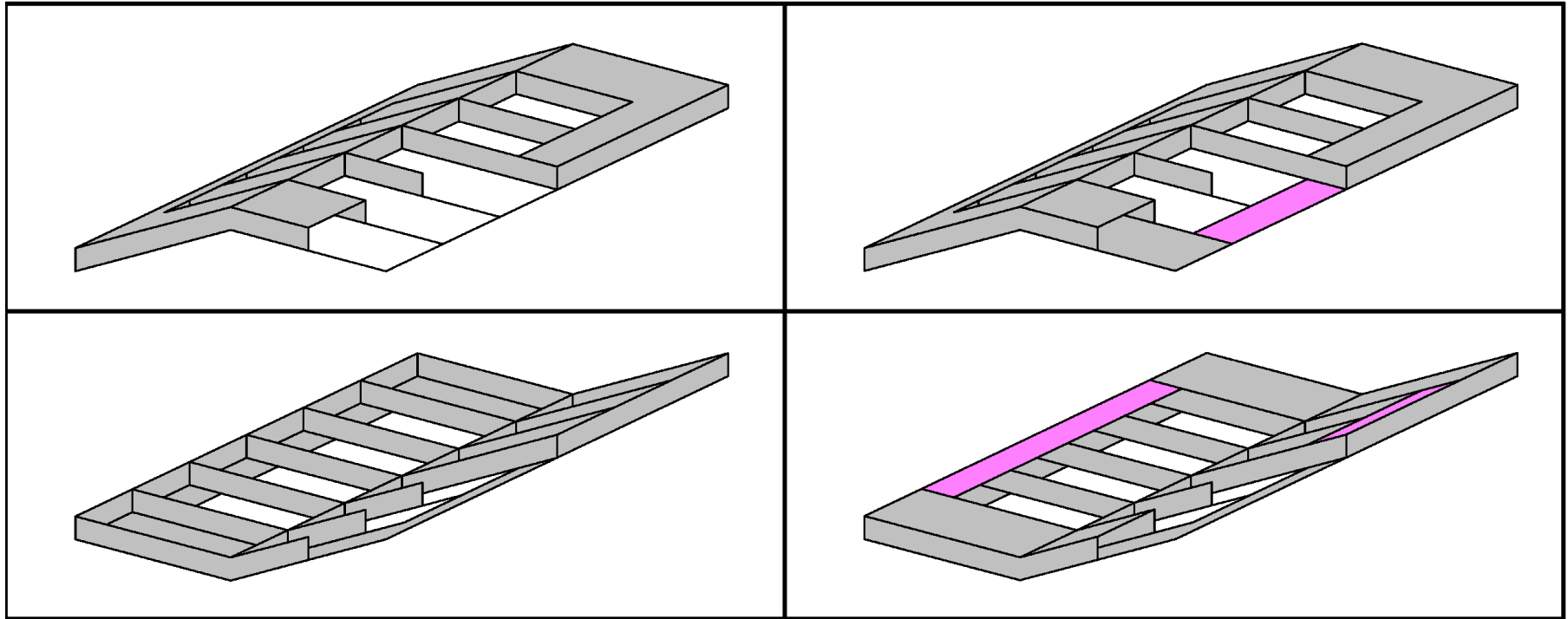
"Heavy" hall - horizontal lower transversal bracings



"Light" hall - no horizontal lower transversal bracings

Photo: Author

"Heavy" hall - horizontal lower longitudinal bracings



"Light" hall - no horizontal lower longitudinal bracings

Photo: Author

## "Heavy" hall, wall bracing - top part of column

For halls with cranes;

Under transverse roof bracings;

Transmission of loads from top part of wall (wind on front wall, prevention of girder buckling and crane loads) to bases;

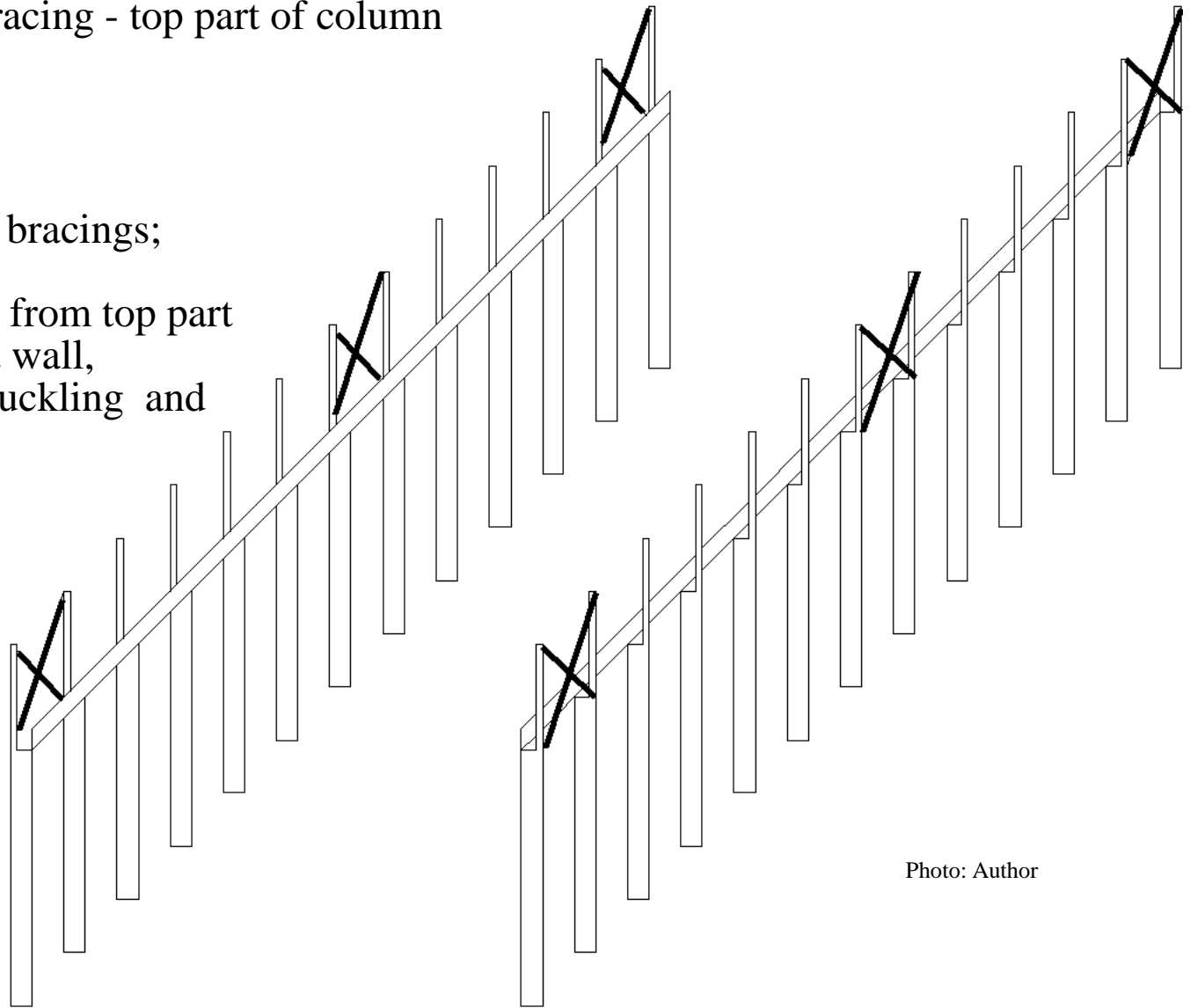


Photo: Author

## "Heavy" hall, wall bracing - bottom part of column

For each types of halls;

Under transverse roof bracings in central part of hall between expansion joints;

Transmission of loads from wall (wind on front wall, prevention of girder buckling and crane loads, imperfections of columns) to bases;

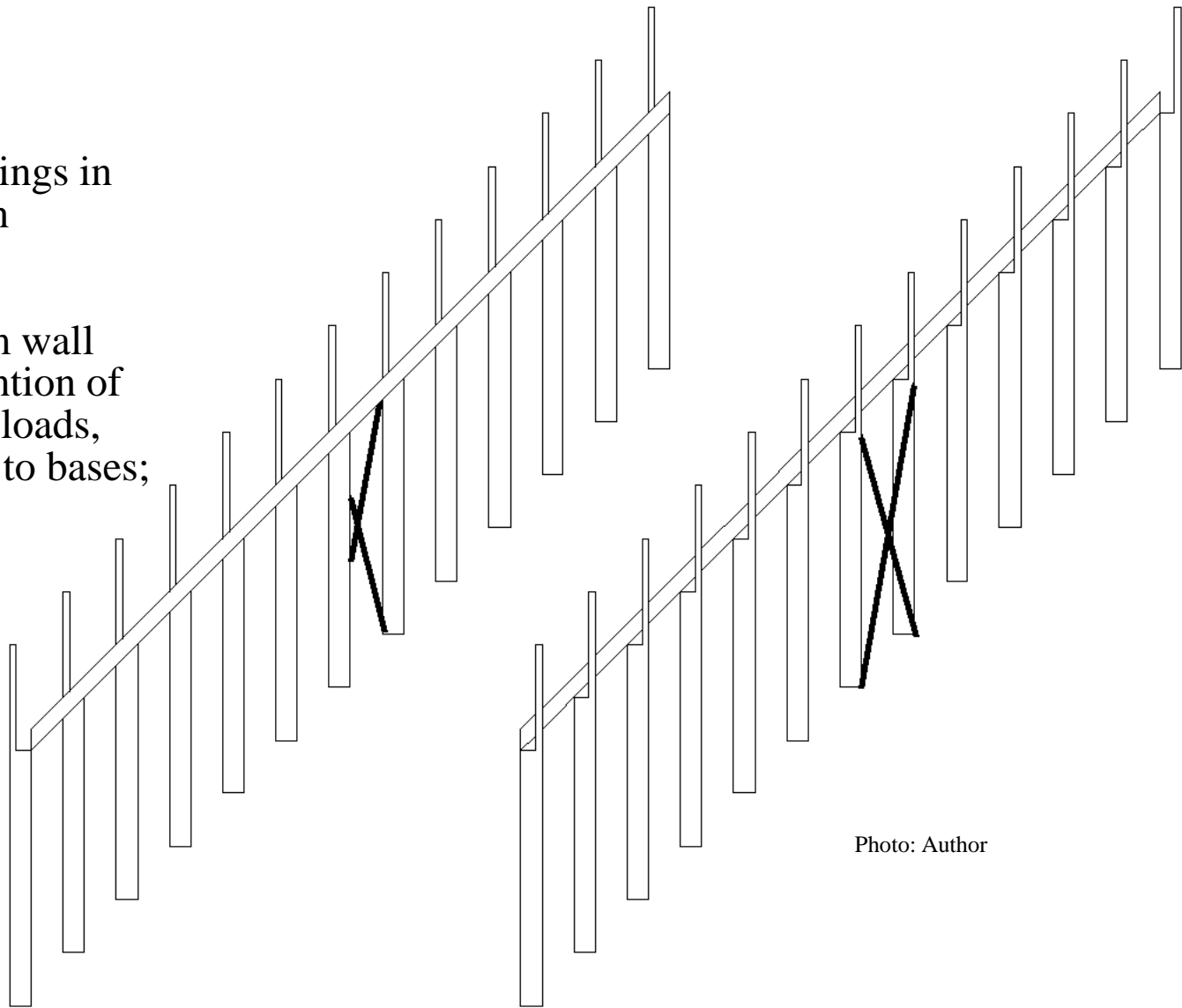


Photo: Author

## "Light" hall, wall bracing - only one part of column

For each types of halls;

Under transverse roof bracings in central part of hall between expansion joints;

Transmission of loads from wall (wind on front wall, prevention of girder buckling, imperfections of columns) to bases;

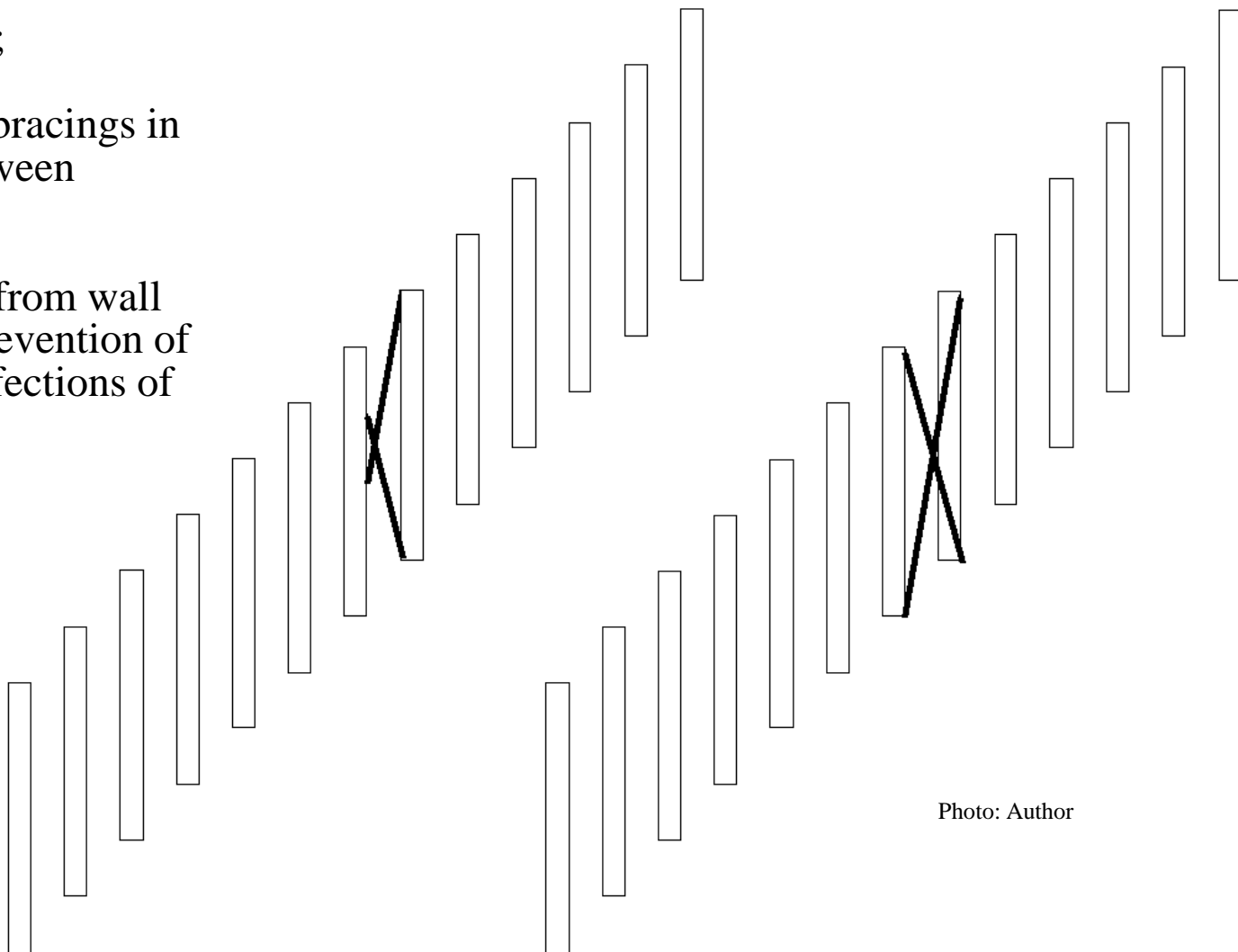


Photo: Author

## Frames



Photo: ubcoholdings.com

$L < 25 - 30 \text{ m} \rightarrow$  hot rolled I-beam IP

$L > 25 - 30 \text{ m} \rightarrow$  welded I-beam IK

$L > 40 - 45 \text{ m} \rightarrow$  roof girder: welded I-beam IK or truss

"Heavy" hall; additional elements for run-beams:



Photo: lindab.com/

Hangers (monorail hoist block, overhead underslung crane)

Photo: udhavind.com



Cantilevers (top-mounted crane)

"Heavy" hall; columns:



Photo: stabud.eu

Massive welded I-beams



Laced

Photo: pebsteel.com



Battened

Photo: hak.com.pl

Tied portal frame

Enormous reduction of bending moments in roof girders and columns, enormous tensile force in tie-beams



Photo: lindab.com

# Portal frame vs. tied portal frame

Two frames, span  $2 \times 15,00$  m, height  $5,00 + 2,50$  m, continuous load  $14 \text{ kN / m}$ :

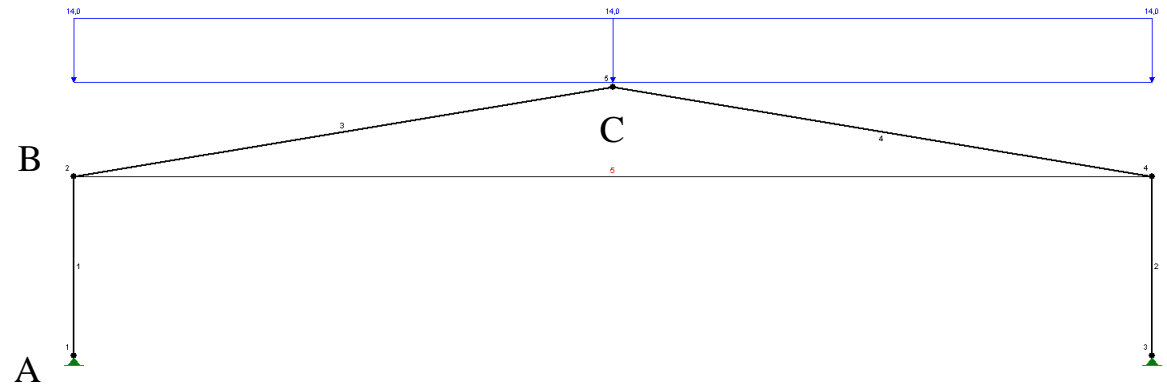
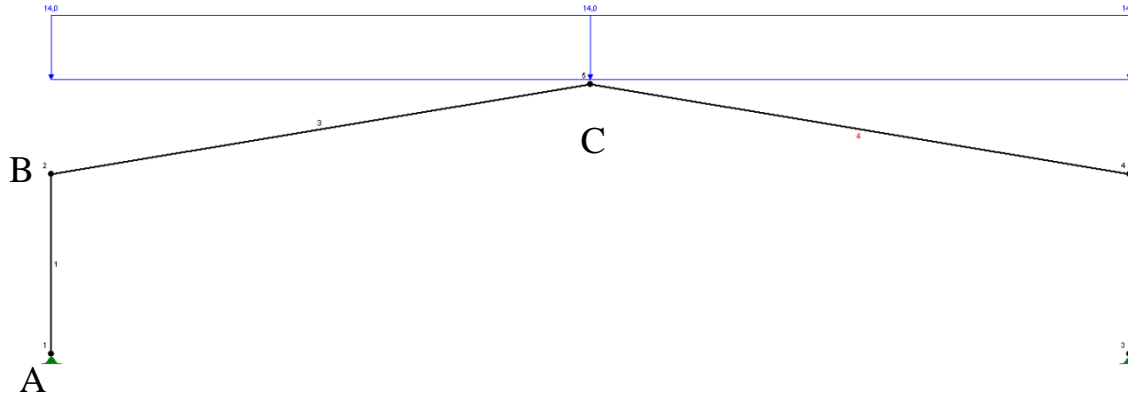


Photo: Author

Member	Point	Portal frame			Tied portal frame		
		$M_{Ed}$ [kNm]	$V_{Ed}$ [kN]	$N_{Ed}$ [kN]	$M_{Ed}$ [kNm]	$V_{Ed}$ [kN]	$N_{Ed}$ [kN]
Roof girder	C	375,8	28,4	170,2	271,1 (0,721)	112,0 (3,944)	708,6 (4,163)
	$M_{opposite}$	403,6			163,2 (0,404)		
	B	862,6	191,2	206,8	237,7 (0,276)	107,6 (0,563)	672,0 (3,250)
Column	B	862,6	172,5	222,6	237,7 (0,276)	47,5 (0,275)	222,6 (1,000)
	A	0,0	172,5	226,8	0,0	47,5 (0,275)	226,8 (1,000)
Tie beam	C						- 633,7

## "Light" hall vs. "heavy" hall

Aspect	„Light” hall	„Heavy” hall
Consequence class	The most often CC2.	Industry hall, for example arms industry → possibility of economic, social or environmental consequences very heavy → CC2 or CC3.
Roofing and housing	Each types of roofing can be applied.	For "heavy" halls we rather not take into consideration anti-buckling protection for purlins by corrugated sheet.
Skylight	<ul style="list-style-type: none"> <li>• There is no special difference between both types of halls for applying of skylights;</li> <li>• We need additional vertical bracings in roof, under edges of skylights.</li> </ul>	
Purlins	Each types of purlins can be applied.	Cold-formed purlins are not recommended for "heavy" halls; especially for purlins cooperating with roof bracings.

Aspect	„Light” hall	„Heavy” hall
Wall girts	There is no special difference between both types of halls for applying of girts.	
Roof bracings	Only „simple” complex of bracings.	Full complex of bracings.
Wall bracings	One part of column.	Top and bottom part of colum.
Main frame	Hot-rolled & welded cross-sections, tied frame and truss roof girders.	<ul style="list-style-type: none"> <li>• Hot-rolled &amp; welded cross-sections and truss roof girders;</li> <li>• Tied frame are not recommended;</li> <li>• Additional elements for suspension / support of run-beams</li> <li>• Columns much more massive (battened, laced, welded).</li> </ul>
Column support	Hinge.	Rigid.
Bolted joint	Static type of actions → bolted joints cathegory A, B, D.	Often dynamic type of actions → bolted joints cathegory C, E.

An important aspect are specific types of snow and wind loading the hall roofs:

Eaves over warehouse ramps

Multi-bay hall → multi-span roof



Photo: bryla.pl



Photo: bestor.com.pl



Photo: krajewski-konstrukcje.pl

Skylights, air-condition, solar thermal collectors, smoke installations, supporting structure for advertising, attics, different height of building parts...

Each of these elements change value of snow loads (drifting) and wind actions

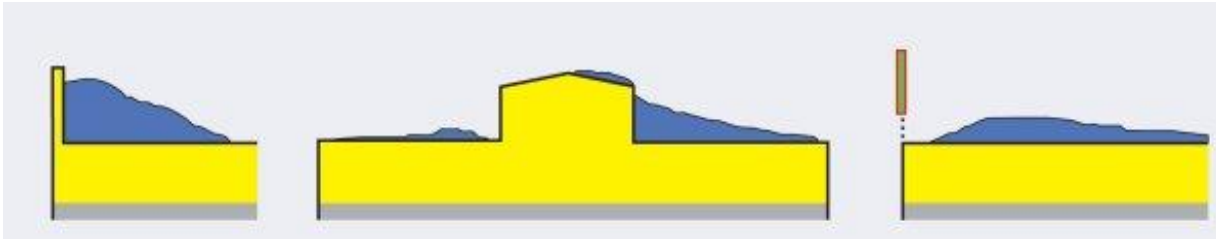


Photo: inzynierbudownictwa.pl

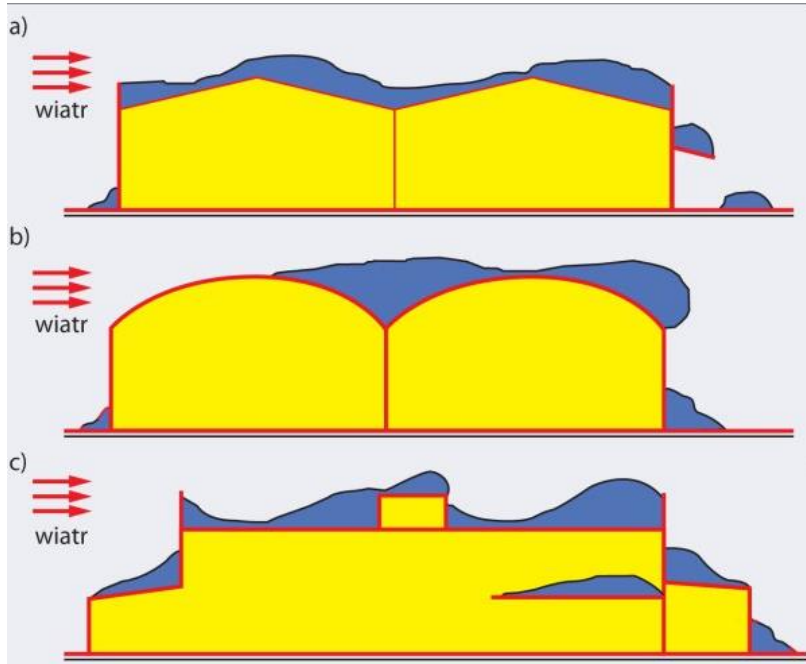
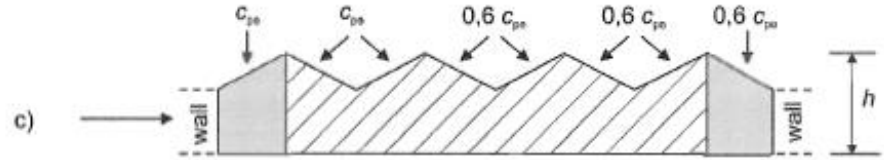
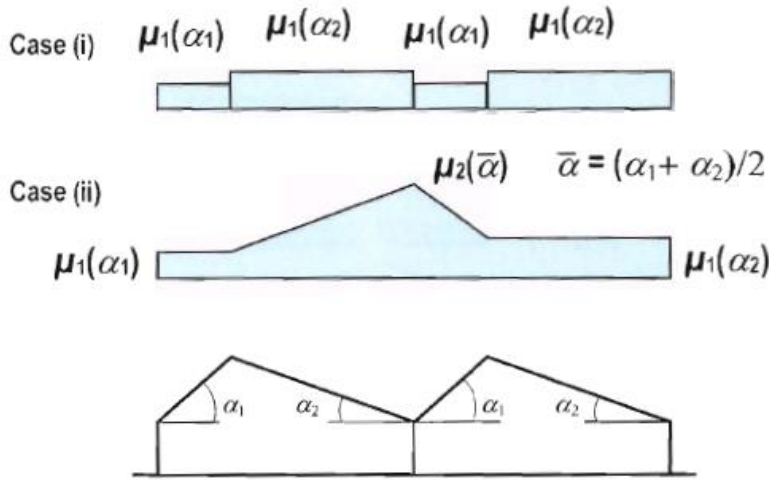


Photo: inzynierbudownictwa.pl



Photo: izolacje.com.pl

# Multi-span roof



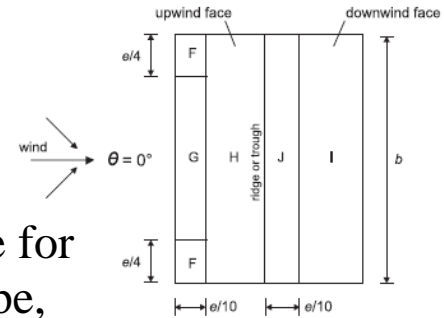
## Snow

Photo: EN 1991-1-3 fig. 5.4

"Prependicular" wind - the same for first, second and third roof slope, 60% for next.

## Wind

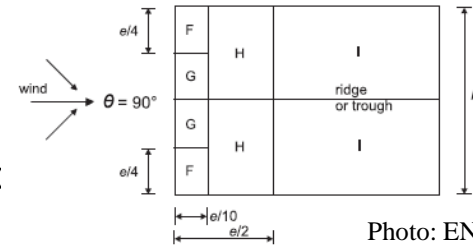
EN 1991-1-7 fig. 7.10



(b) wind direction  $\theta = 0^\circ$

$e = b$  or  $2h$   
whichever is smaller  
 $b$  : crosswind dimension

"Parallel" wind - no difference for one-span and multi-span roof



(c) wind direction  $\theta = 90^\circ$

Photo: EN 1991-1-4 fig. 7.7

# Snow - attic, skylight, different height, additional elements

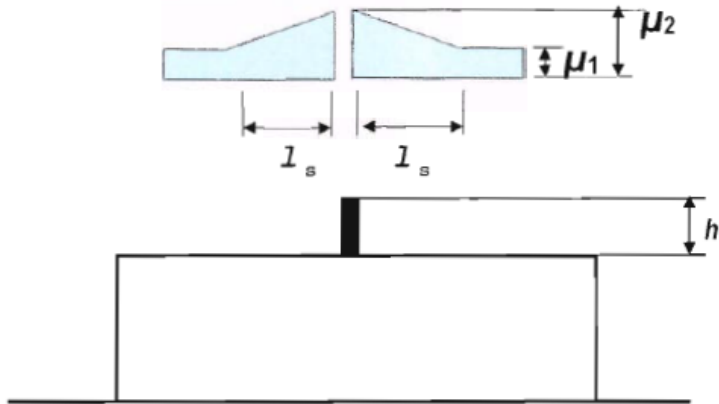


Photo: EN 1991-1-3 fig. 6.1

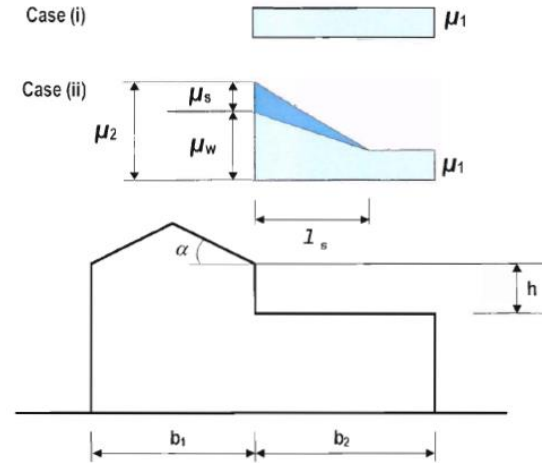
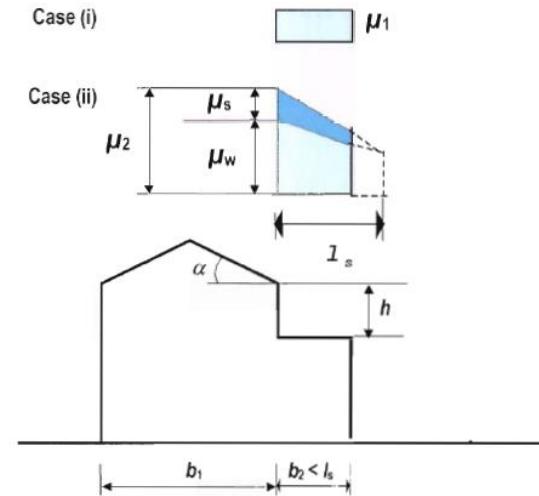
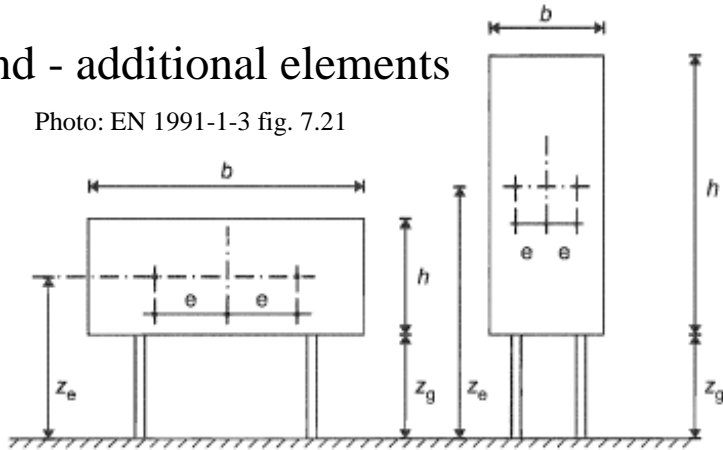


Photo: EN 1991-1-3 fig. 5.7



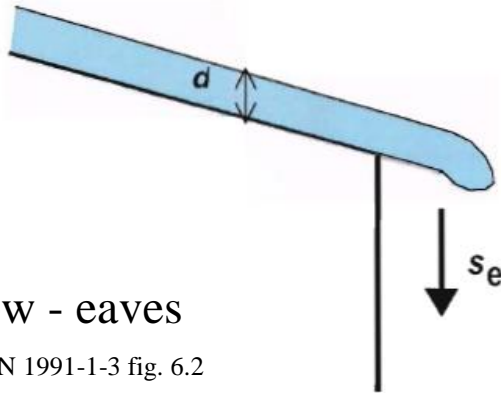
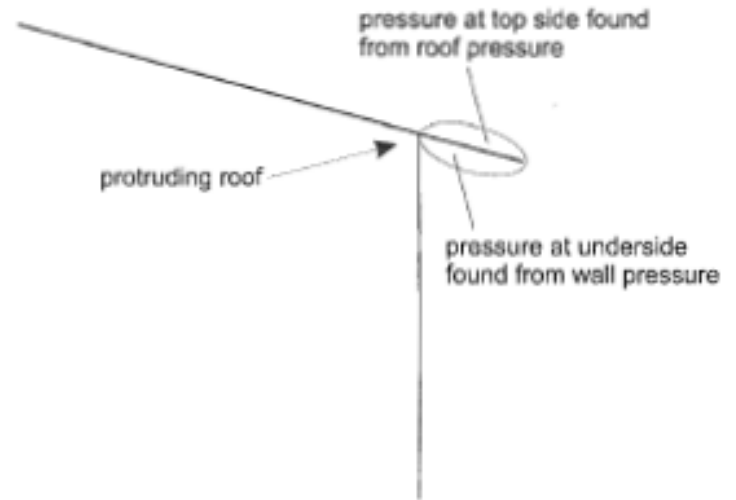
# Wind - additional elements

Photo: EN 1991-1-3 fig. 7.21



NOTE 1 reference height:  $z_e = z_g + h/2$

NOTE 2 reference area:  $A_{ref} = b \cdot h$



## Snow - eaves

Photo: EN 1991-1-3 fig. 6.2

## Wind - attic and eaves

Photo: EN 1991-1-3 fig. 7.3

# Long span structures

Lec #6

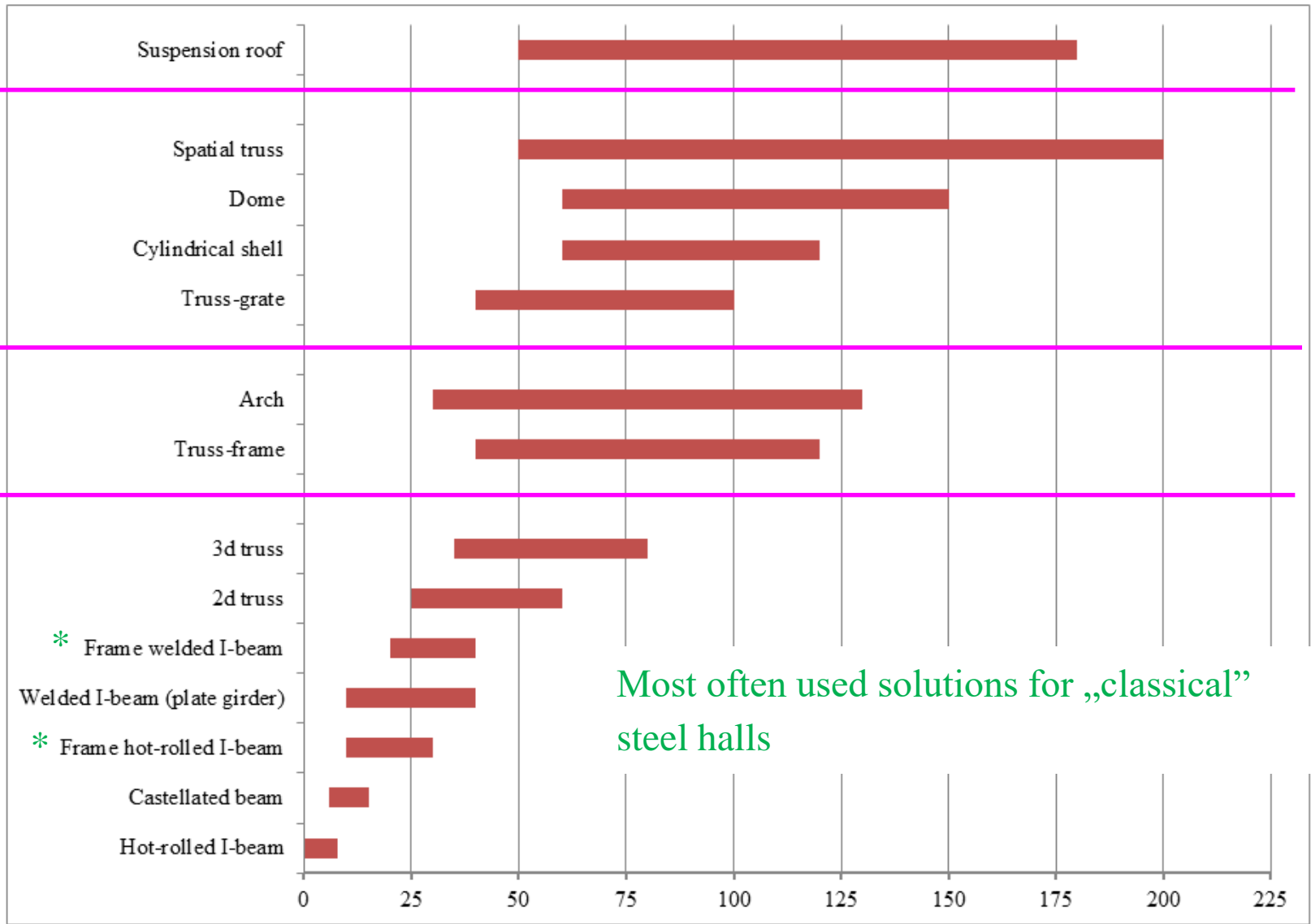
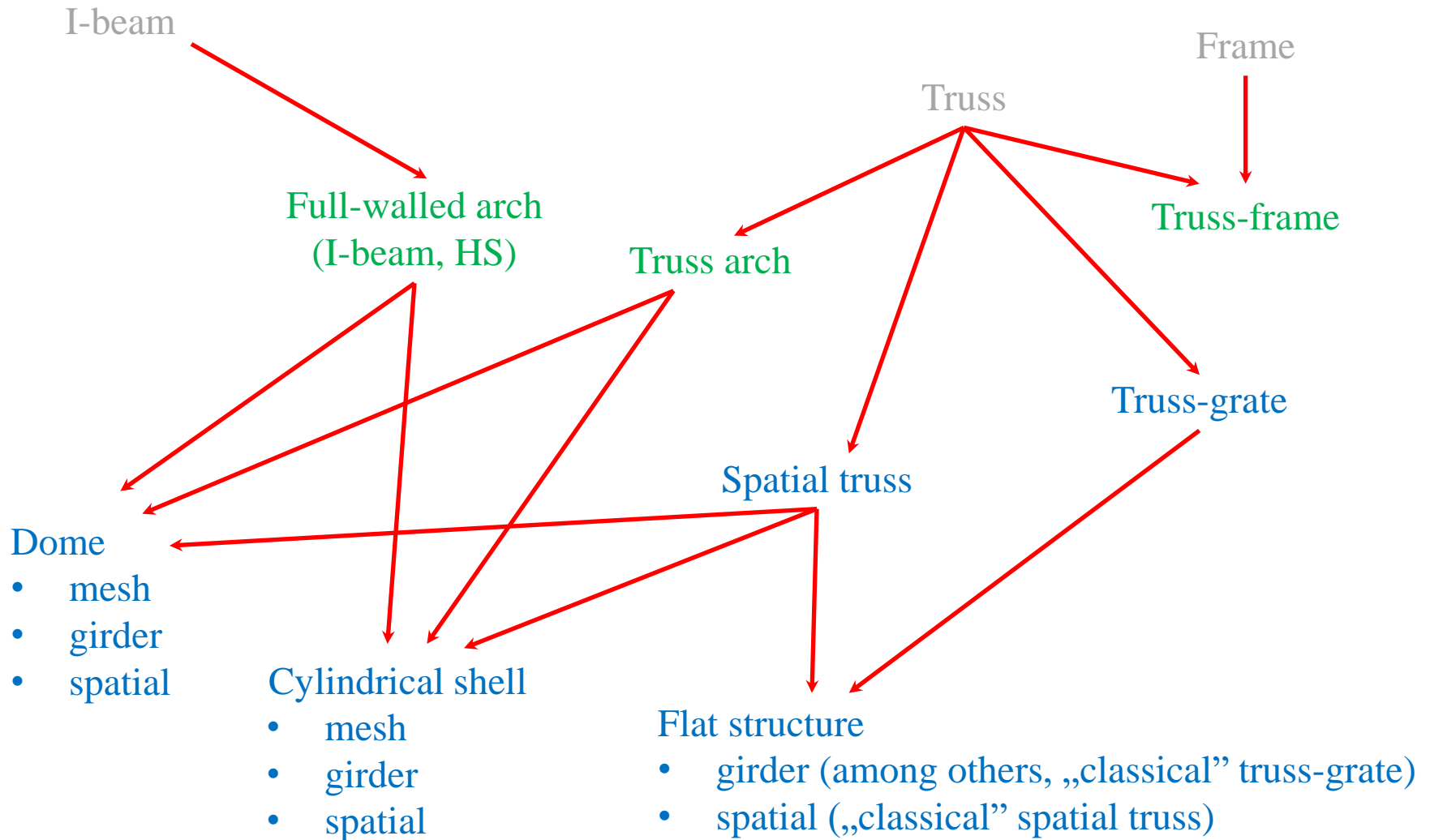


Photo: Author

Recommended length for different types of long span structures.

# Simple, complex and advanced



## One span I-beam

$$h = L (1/20 \sim 1/25)$$



Photo: Author

Requirements:

$$V_{Ed} / V_{Rd} \leq 1,0$$

$$M_{Ed} / M_{Rd} \leq 1,0$$

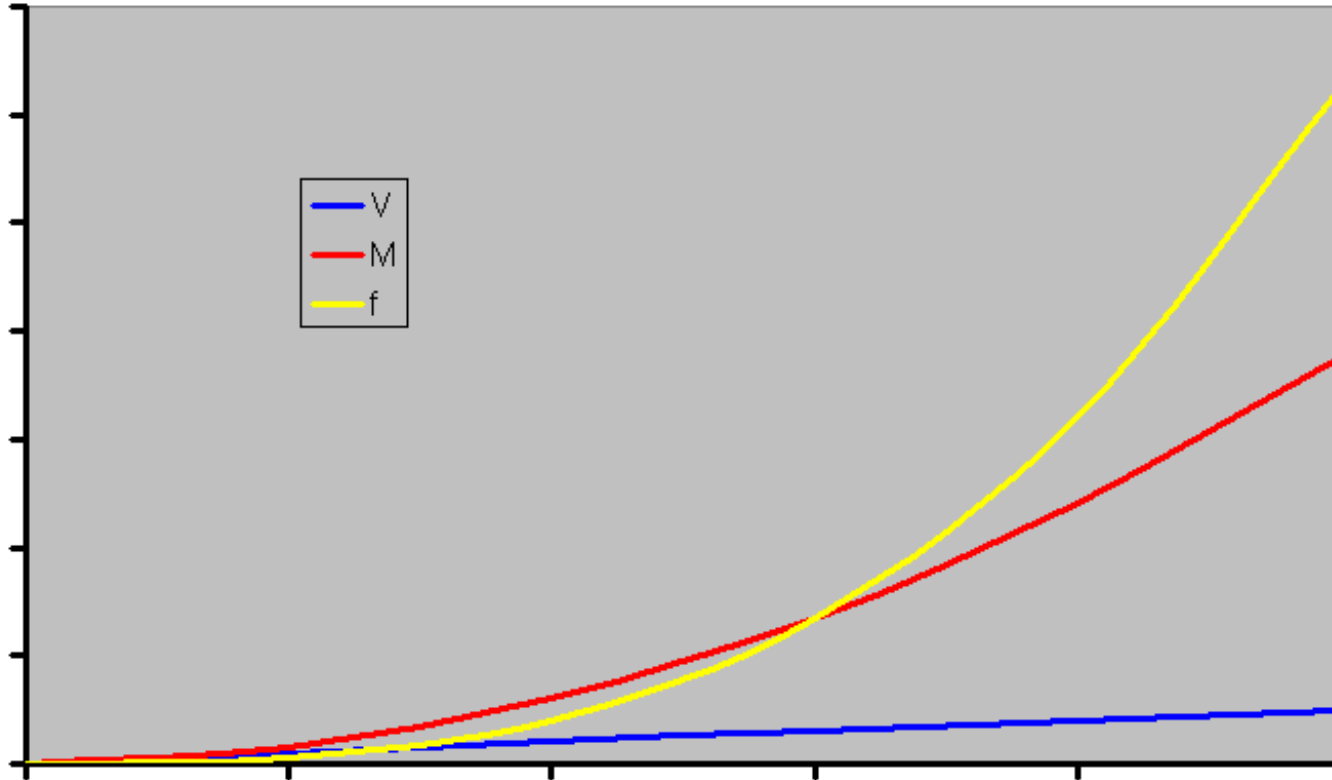
$$f / f_{sls} \leq 1,0$$

$$V_{Ed} / V_{Rd} \leq 1,0 \rightarrow (q L / 2) / (A_v f_y / \sqrt{3}) = L [q A_v f_y / (2 \sqrt{3})] = L a$$
$$L a \leq 1,0$$

$$M_{Ed} / M_{Rd} \leq 1,0 \rightarrow (q L^2 / 8) / (W f_y) = L^2 [q / (8 W f_y)] = L^2 b$$
$$L^2 b \leq 1,0$$

$$f / f_{sls} \leq 1,0 \rightarrow [5 q L^4 / (384 E J)] / (L / \alpha) = (L^4 / L) [5 q \alpha / (384 E J)] = L^3 c$$
$$L^3 c \leq 1,0$$

Photo: Author



Regardless values of  $a$ ,  $b$ ,  $c$ ,

- for very short elements the most important is condition for  $V_{ed}$ ;
- for medium length of elements the most important is condition for  $M_{ed}$ ;
- for long elements elements the most important is condition for  $f$ ;



Generally, condition for  $V_{ed}$  is important for very short cantilevers only.

Photo: udhavind.com

$L = 0 \sim 6 \text{ m} \rightarrow$  more important is condition  $M_{Ed} / M_{Rd} \leq 1,0$

$L \geq \sim 6 \text{ m} \rightarrow$  more important is condition  $f / f_{sls} \leq 1,0$

## One span castellated beam

$$h_{\text{start}} = L (1/20 \sim 1/25)$$

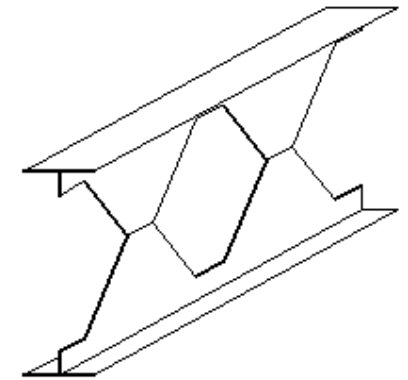
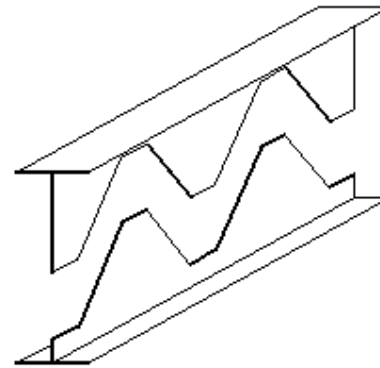
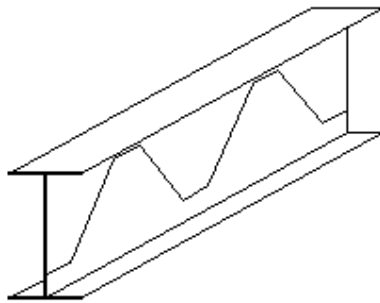
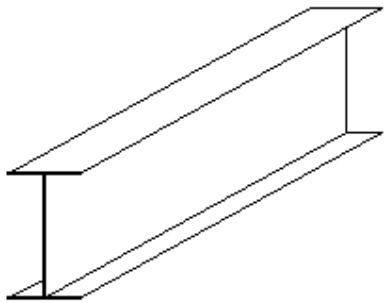


Photo: gunungsteel.com

Dead weight (I-beam)  $\approx$  dead weight (castellated)

J (castellated)  $\gg$  J (I-beam)

Photo: Author





## Plate girder

$$h = L (1/10 \sim 1/16)$$

IPE, HE, HL - constant dimensions of cross-section

Plate girder - the possibility to change dimensions of cross-section

Maximum high:

IPE 750: 753 - 770 mm

HE 1000: 970 - 1055 mm

HL 1100: 1090 - 1118 mm

...

Plate girder - no limit



Photo: r4engineers.com

## Truss (2D)

$$h = L (1/10 \sim 1/15)$$

Photo: doubletreestructures.com



Photo: waldenstructures.com



## Truss (3D) = multi-chords truss



Photo: steelconstruction.info

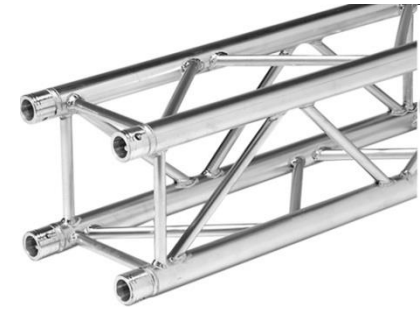


Photo: conference-truss-hire.co.uk



Photo: multimetalgb.ca

## Frame (hot-rolled I-beam)

$$h = L (1/40 \sim 1/50)$$



Photo: setrometalgroup.com

## Frame (welded I-beam)

$$h = L (1/30 \sim 1/40)$$



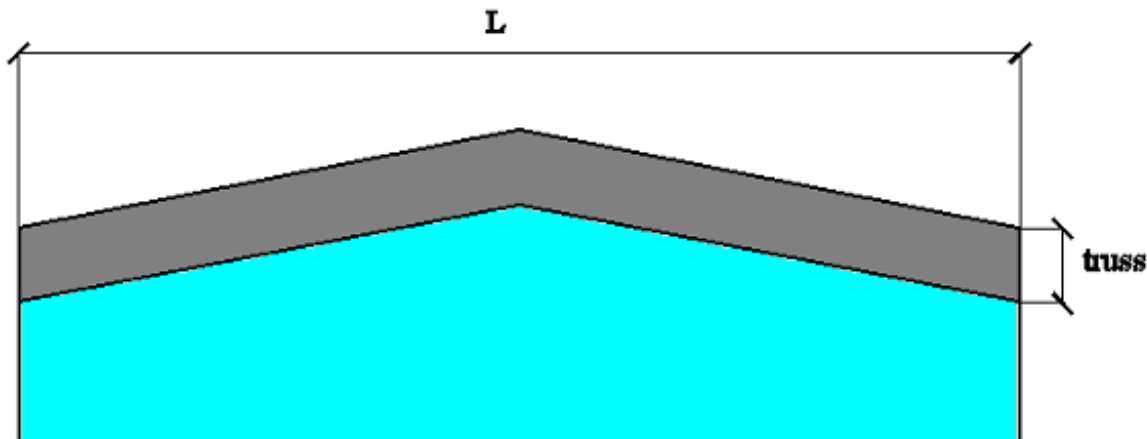
Photo: traskostal.pl

# Frame (I-beam) ↔ truss → economy

Photo: doubletreestructures.com



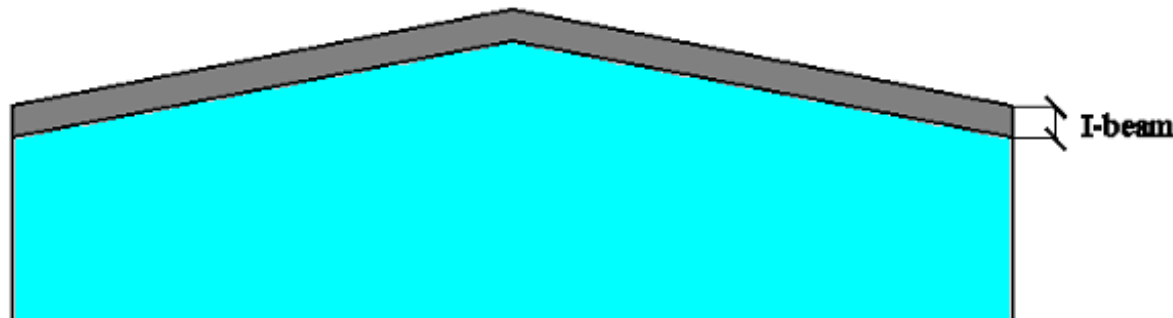
Photo: traskostal.pl



$$H_{\text{truss}} = L (1/10 \sim 1/15)$$

$$H_{\text{frame}} = L (1/40 \sim 1/50)$$

$$H_{\text{frame}} \ll H_{\text{truss}}$$



There is more usefull space for frame structure than for truss structure.

Photo: Author

# Arch

$$f = L (1/1 \sim 1/80)$$

Photo: lusas.com



Photo: dziennikpolski24.pl

Reactions:

Arch  $\leftrightarrow$  Arch + tie-beam

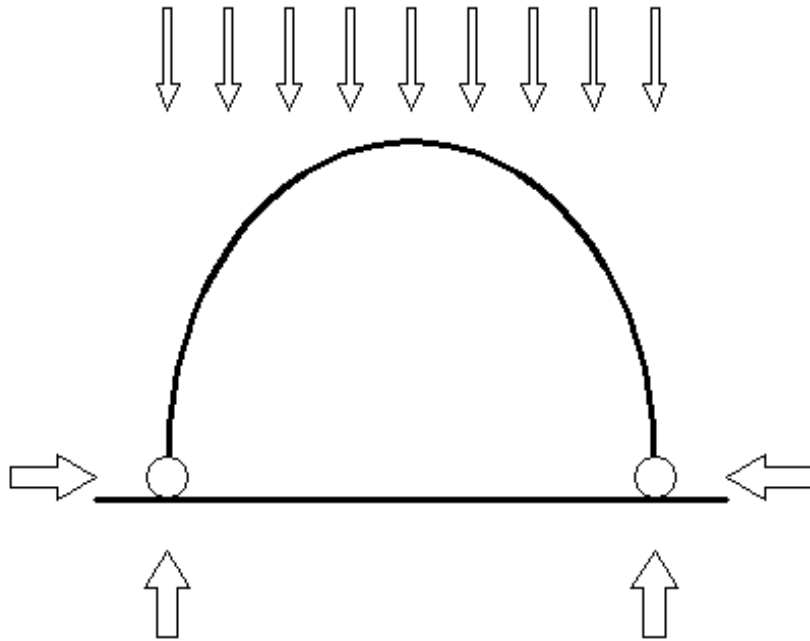
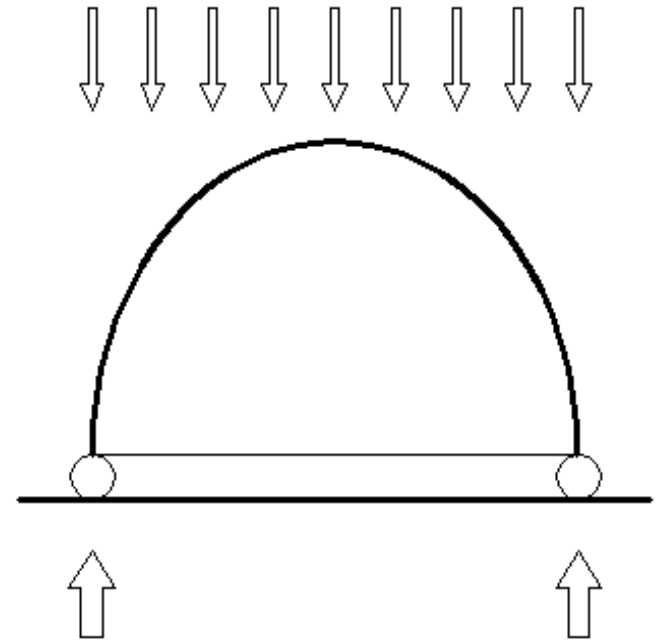


Photo: Author



## Frame-truss

Idea of „classical” frame, made on truss

$$h = L (1/12 \sim 1/20)$$



Photo: wikipedia

## Mesh structure (net structure)

Bar structure; curved surface formed from triangular arrangement of bars. Cross-section of each bar is generally the same. Stiffness comes from curvature of surface and triangular arrangement.



Photo: wellter.com.cn



Photo: hindustanalcoxlimited.wordpress.com

Schwedler's dome - meridional and latitudinal bars, connected by diagonal ones.

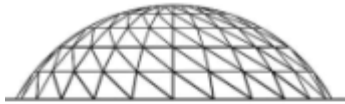


Photo: chodor-projekt.net

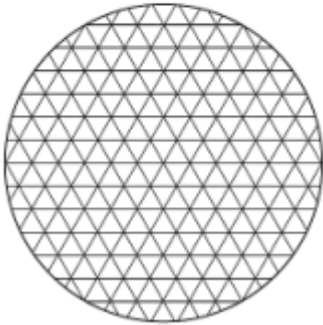
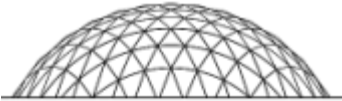
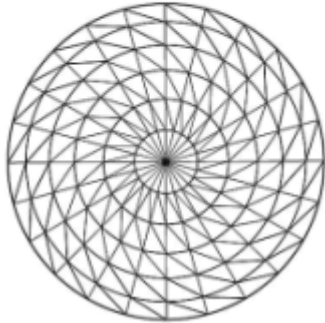


Photo: chodor-projekt.net

Three-way dome - arrangement of bars in three directions, at angles of  $60^\circ$  to each other. When viewed from above, it forms equilateral triangles. In fact, the closer to base, the more triangles differ from each other.

This solution could be applied to cylindrical shells.

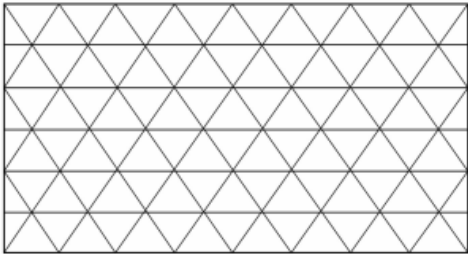


Photo: chodor-projekt.net

Lamella dome (stright or curved) - latitudinal bars (circles) with an arrangement of bars in two other directions (straight or curved bars).

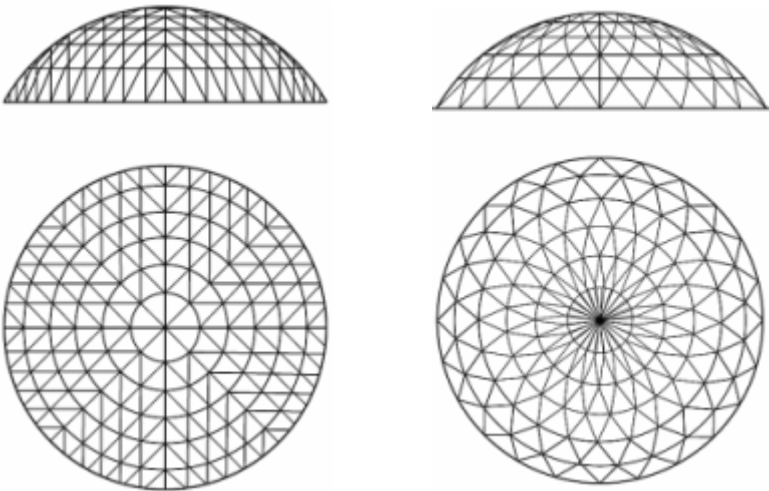


Photo: chodor-projekt.net

Solution also used for cylindrical shells: Lamella (rhombic arrangement) or Lamella-Föppl (triangular arrangement).

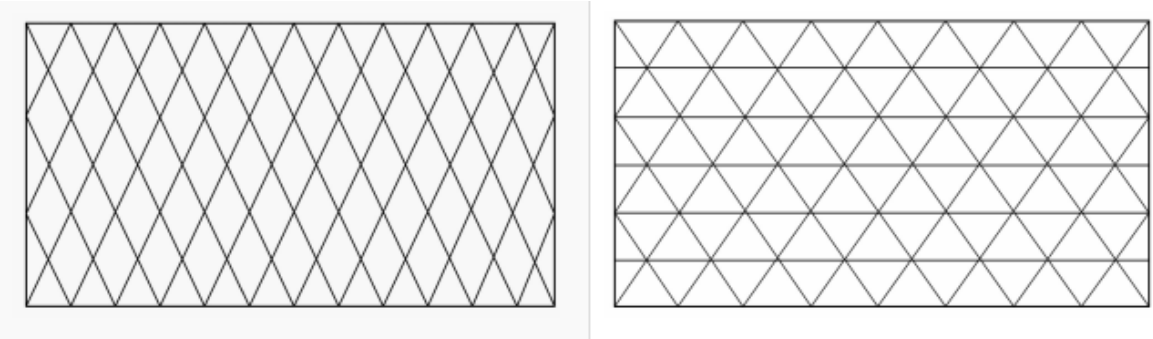


Photo: chodor-projekt.net

Geodesic dome - all triangles forming the mesh are equilateral or almost equilateral. One of the most often applied solution.

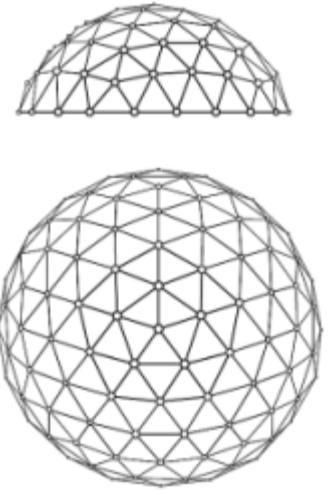


Photo: chodor-projekt.net

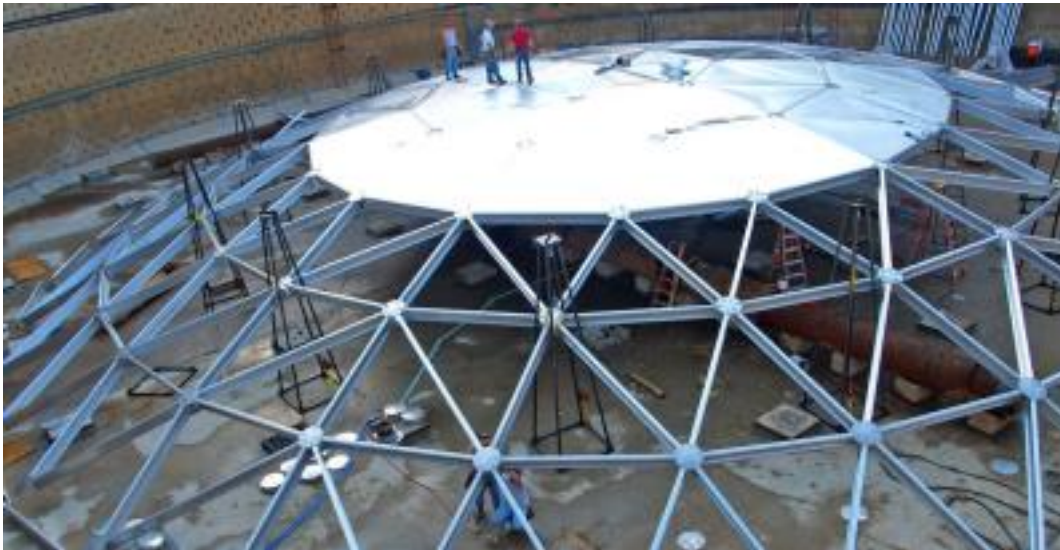


Photo: eeaustin.com

For cylindrical shells, orthogonal systems are applied: longitudinal and transversal bars connected by diagonal ones.

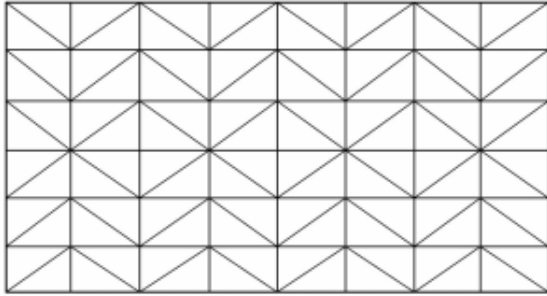


Photo: chodor-projekt.net

Warren sub-type (diagonal bars must be placed in such a way as to avoid compression)

Pratt sub-type

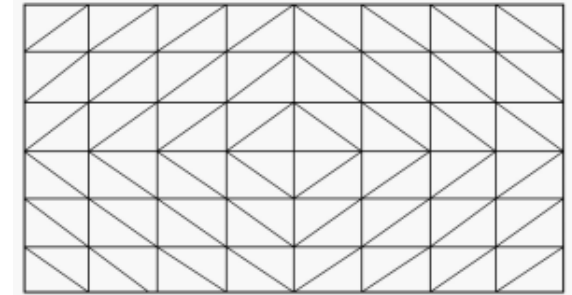


Photo: chodor-projekt.net

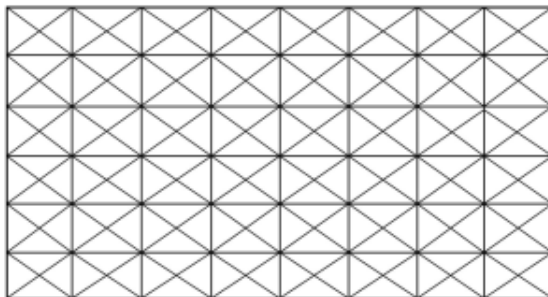


Photo: chodor-projekt.net

X sub-type

## Girder structure

Bar structure; main elements are girders (full-walled or trussed), one- or two-way, supplemented by system of secondary beams.



Photo: wasatchsteel.com

One-way girders, cylindrical shell and dome.



Photo: wikipedia



Photo: videohive.net



Girder dome (one-way),  
truss arches



Photo: lfspaceframe.com

# Truss-grates

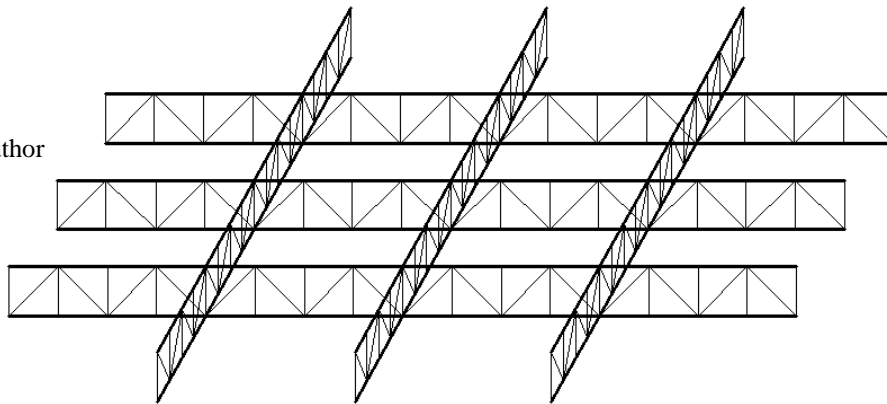


Photo: [cdn8.muratorplus.smcloud.net](http://cdn8.muratorplus.smcloud.net)



Photo: [qdjinfei.en.made-in-china.com](http://qdjinfei.en.made-in-china.com)

Photo: Author



Complex of trusses of the same height, perpendicular each other (two-way girders): flat structures, cylindrical shells, domes.

Steel grate (two-way girders, flat structure), full-walled girdes



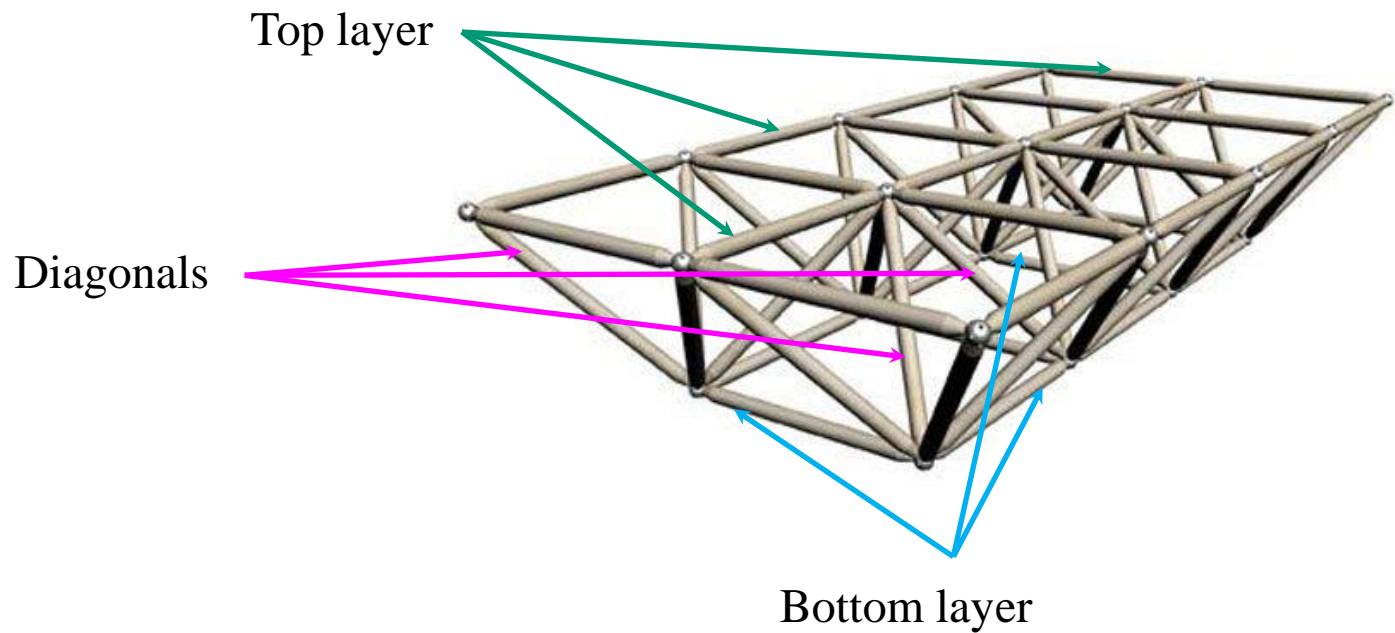
Photo: [steelbuildingstructure.com](http://steelbuildingstructure.com)

Photo: [dunkerley.co.uk](http://dunkerley.co.uk)

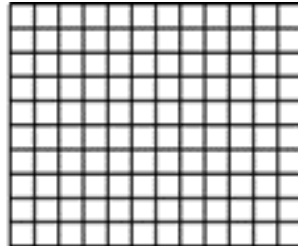
## Spatial structure

Bar structure; two outer layers, connected by truss bars in pyramidal (triangular) arrangement (analogy to "classical" truss: two chords connected by truss bars in triangular arrangement). Cross-section of each bar is generally the same. Stiffness comes from pyramidal arrangement.

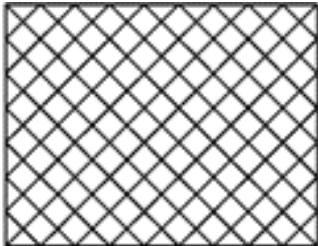
Photo: cnzlf.com



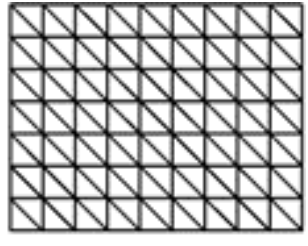
In general, square grids are used in both outer layers and differ from each other by various relative positions (overlap, offset, 45° rotation)



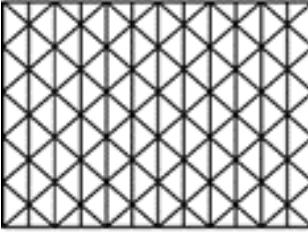
(a) Two-way grid



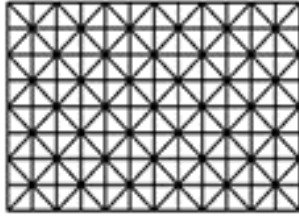
(b) Diagonal grid



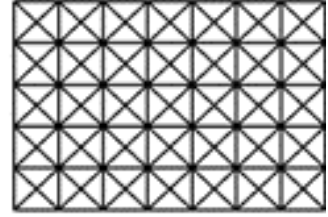
(c) Three-way grid



(d) Three-way grid

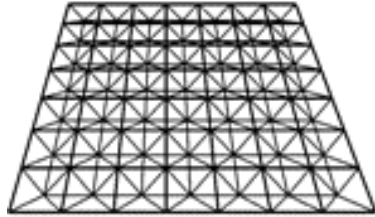


(e) Four-way grid

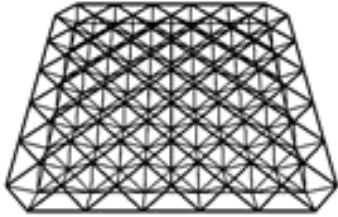


(f) Four-way grid

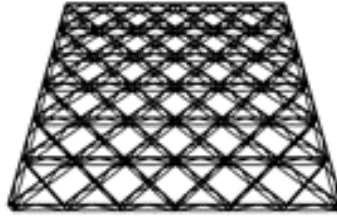
Photo: civiltech.ir



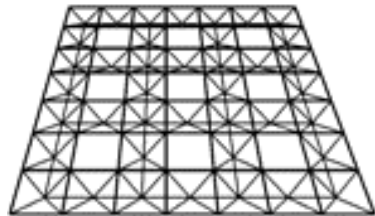
(a) Two-way on two-way grid



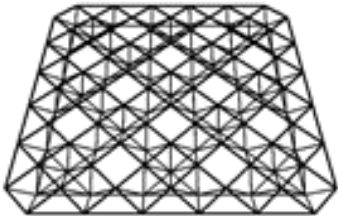
(b) Diagonal on diagonal grid



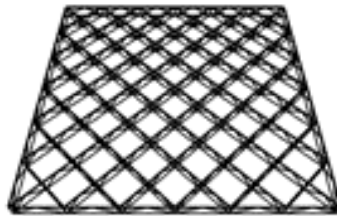
(c) Three-way truss grid



(d) Reduced two-way on two-way grid



(e) Reduced diagonal on diagonal grid



(f) Diagonal truss grid

Photo: civiltech.ir

Spatial structures can be applied to flat surfaces, cylindrical shells and domes. Other name: multi-layers structures.

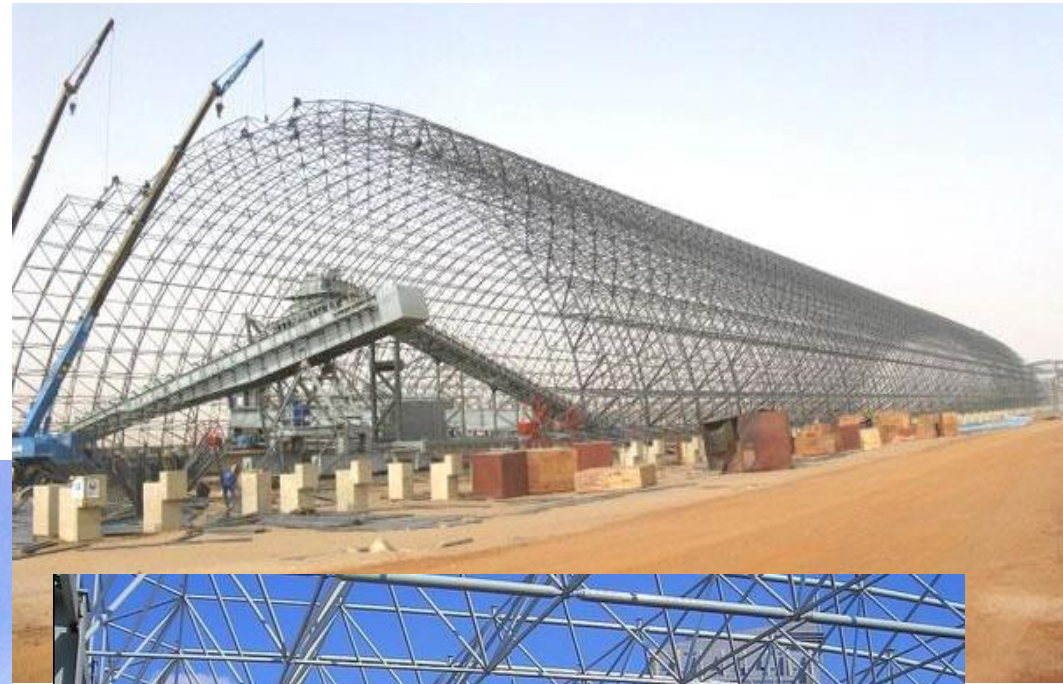
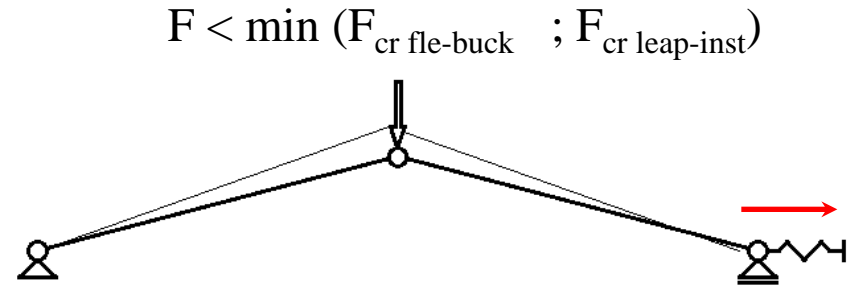
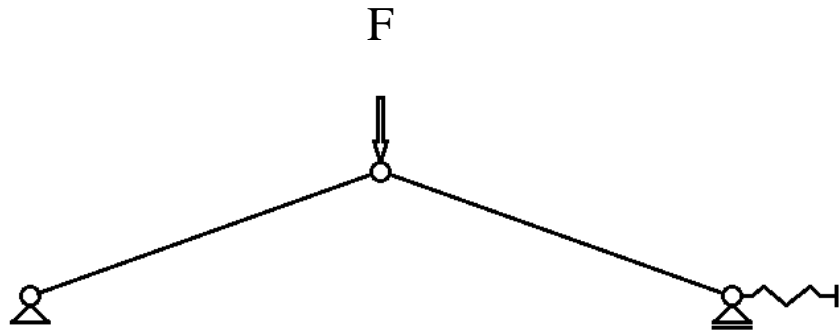


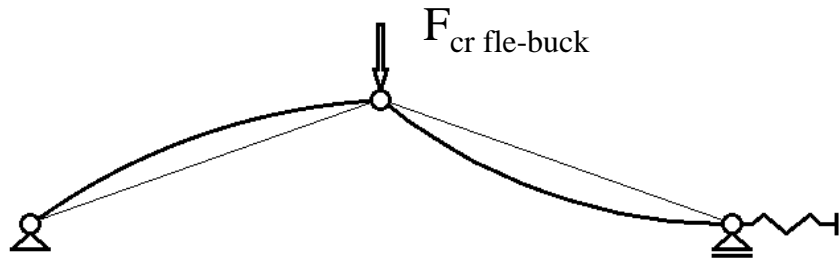
Photo: wikipedia

Photo: miripiri.co.in

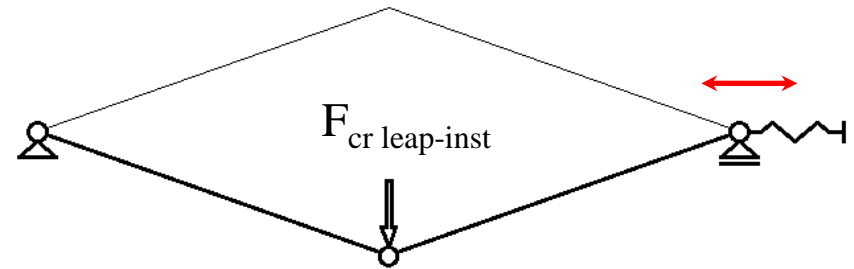
Special type of instability:



Deformation



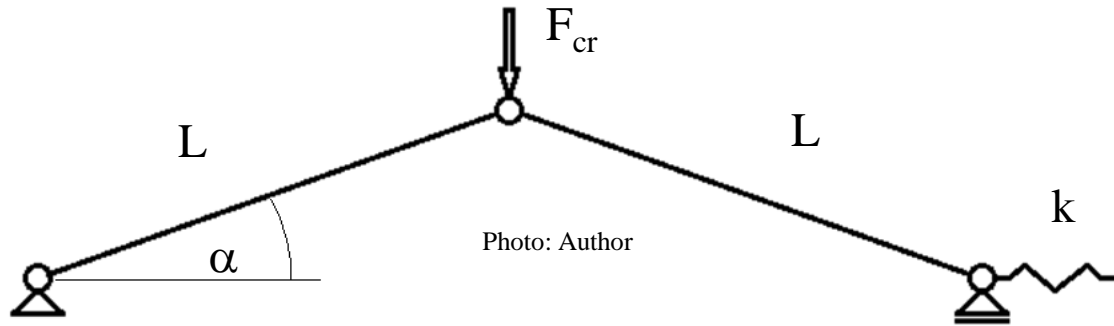
Flexural buckling of compressed bars



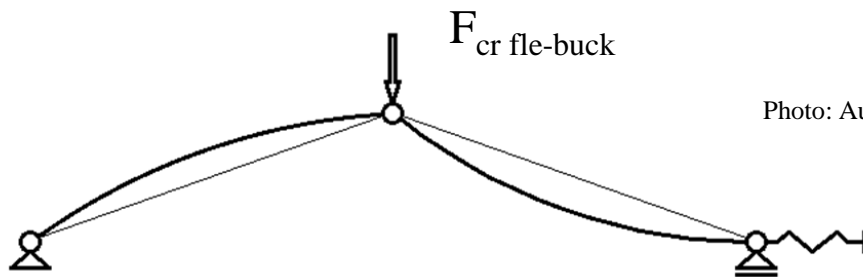
Leaping instability

Photo: Author

Behavior of such structure depends on its span  $L$ , initial angle  $\alpha$  and spring factor  $k$ .



For big values of  $L$ ,  $\alpha$  and  $k$ , more probable is flexural buckling ( $F_{cr \text{ fle-buck}} < F_{cr \text{ leap-inst}}$ ).  
 For small values, more probable is leaping instability ( $F_{cr \text{ leap-inst}} < F_{cr \text{ fle-buck}}$ ).



Flexural buckling of compressed bars

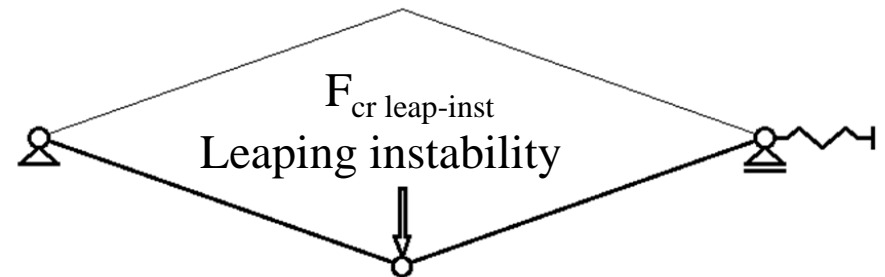




Photo: gadzety-reklamowe.com



Photo: wroclaw.pl

The same as for simplified model, leaping instability for spatial trusses, cylindrical truss shells and domes potentially can occur for critical load smaller, than critical load for flexural instability of bars. Spring in simplified model represents, first of all, stiffness resulting from geometry of steel structure. One of the most important parameter is distance between upper and lower surface of bar nets. Increasing  $H$  means increasing spring factor  $k$ .

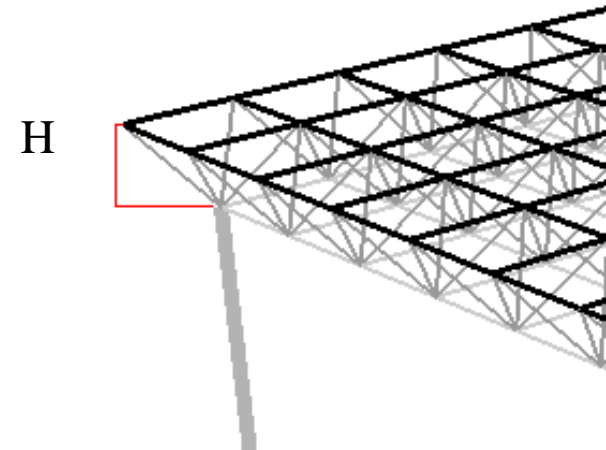


Photo: geometrica.com



Photo: steelstructureschina.com

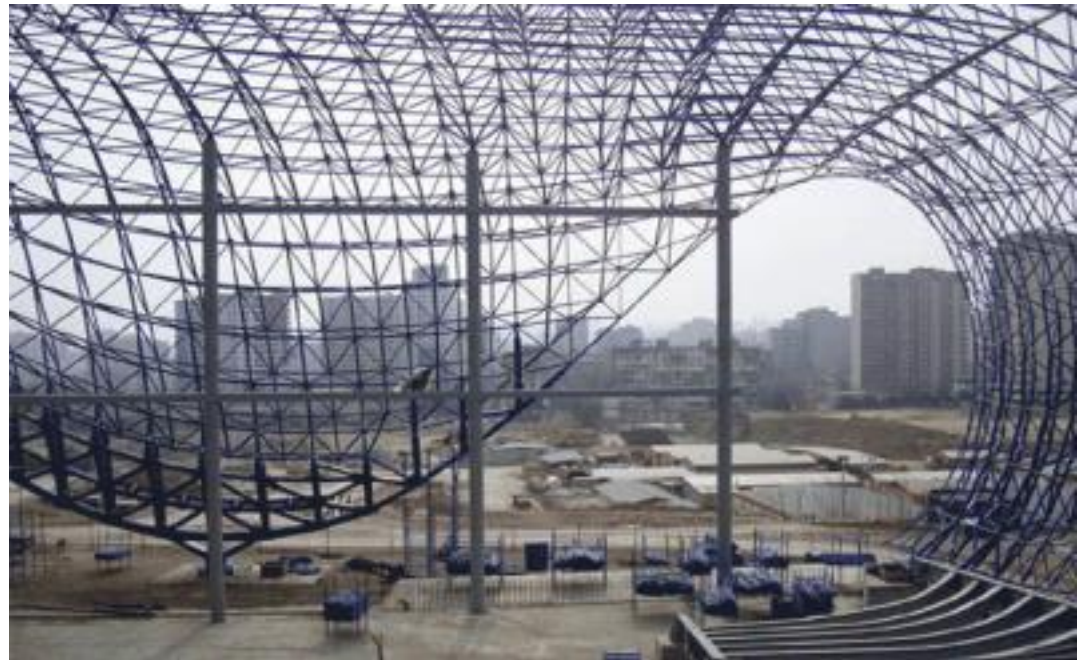


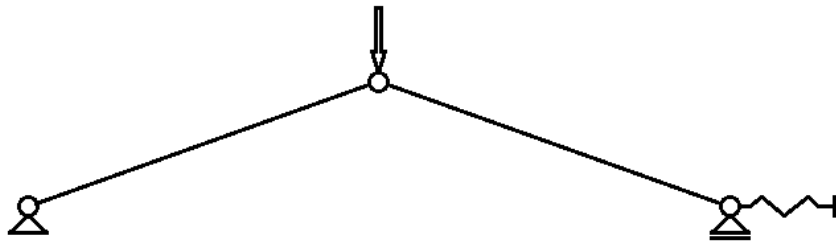
Photo: cnxzf.com



Photo: Author

Other very important parameters are number and stiffness of columns (supports) and shape of surface. Curvature of surface also make impact on factor  $k$ .

Leaping instability isn't easy to analyze. „Classical" analysis - geometrically linear – of instability will reveal only buckling of bars. If  $F_{cr \text{ leap-inst}} < F_{cr \text{ fle-buck}}$  it is a dangerous situation for structure (too optimistic definition of resistance).



Detection of leaping is only possible in geometrically nonlinear stability analysis.

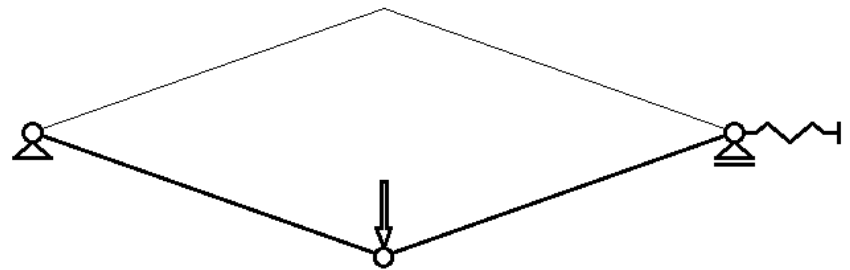
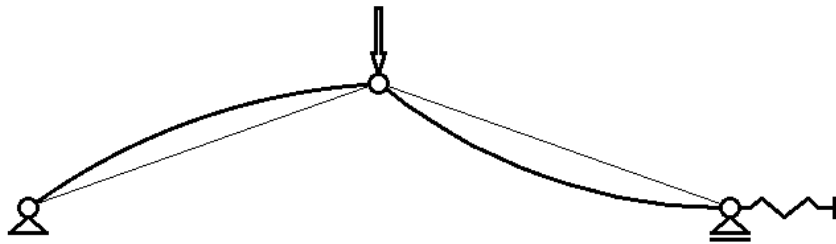


Photo: Author

Spatial structure:

Flat

Cylindrical

Dome

Prefabrication systems:

Mero

SDC

Pyramitec

Unibat

Tesep

Tridimatec

...

# Mero system - the most popular



Photo: pic2fly.com

Photo: signs.pl



Photo: pic2fly.com

SDC, pyramitec, cirotec, bacotec, tridimatec, unibat, spherobat, tridiblau, flotau, pyramibat, bamboutec, dodecavis - systems by Polish architect Stefan du Chateau

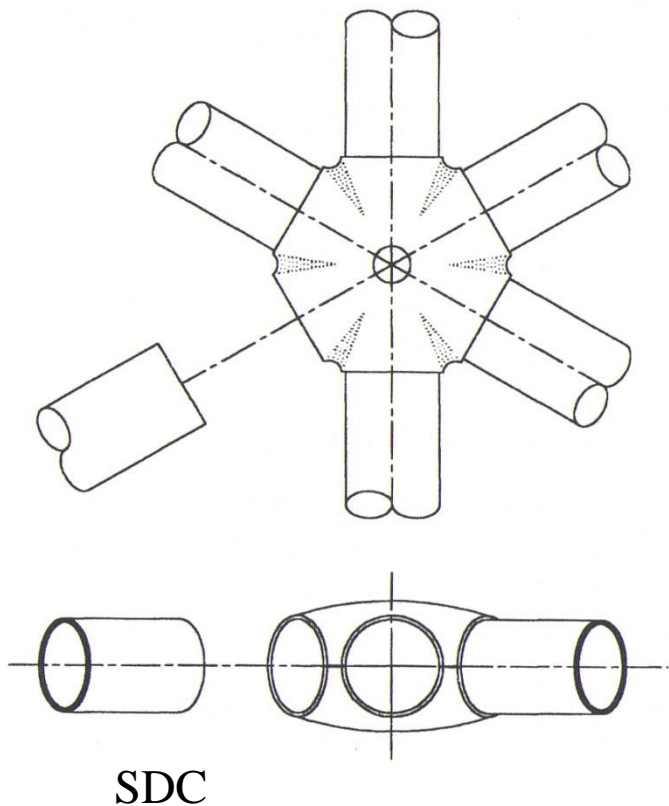
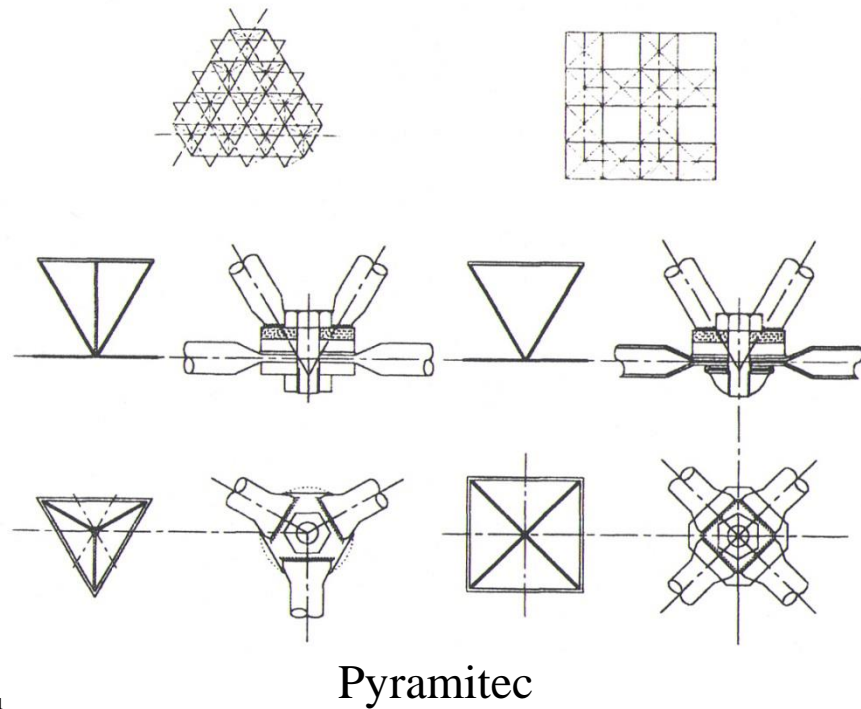


Photo: vt.edu



There is information in EN 1993-1-8 7.1:

deformation ends of element are not accepted.



Photo: tatasteelconstruction.com

This information is not important for prefabricated systems. For this situation, resistance of joints is tested experimentally, not only calculated during design process.

## Examples

- Exhibition halls
- Sport facilities
- Entertainment centres
- Railway stations & airport halls
- Special industrial facilities
- Special transport facilities
- Landmarks
- Bridges

Comparison of dimensions:

Kraków Main Square: 200x200 m;

Hejnalica: 82 m;

Distance: Cracow University of Technology - Wawel Hill: 2 000 m;

## Neue Messe Central Hall, Leipzig (Glass Palace)

Exhibition centre, former  
railway station

length 238 m  
width 80 m  
height 28 m

Arches and cylindrical shell



Photo: e-architect.co.uk

Kraków Main Square: 200x200 m;

## Montreal Biosphère Museum

Former EXPO '67 pavilion,  
today tourist attraction

diameter 76 m  
height 62 m

Spherical dome

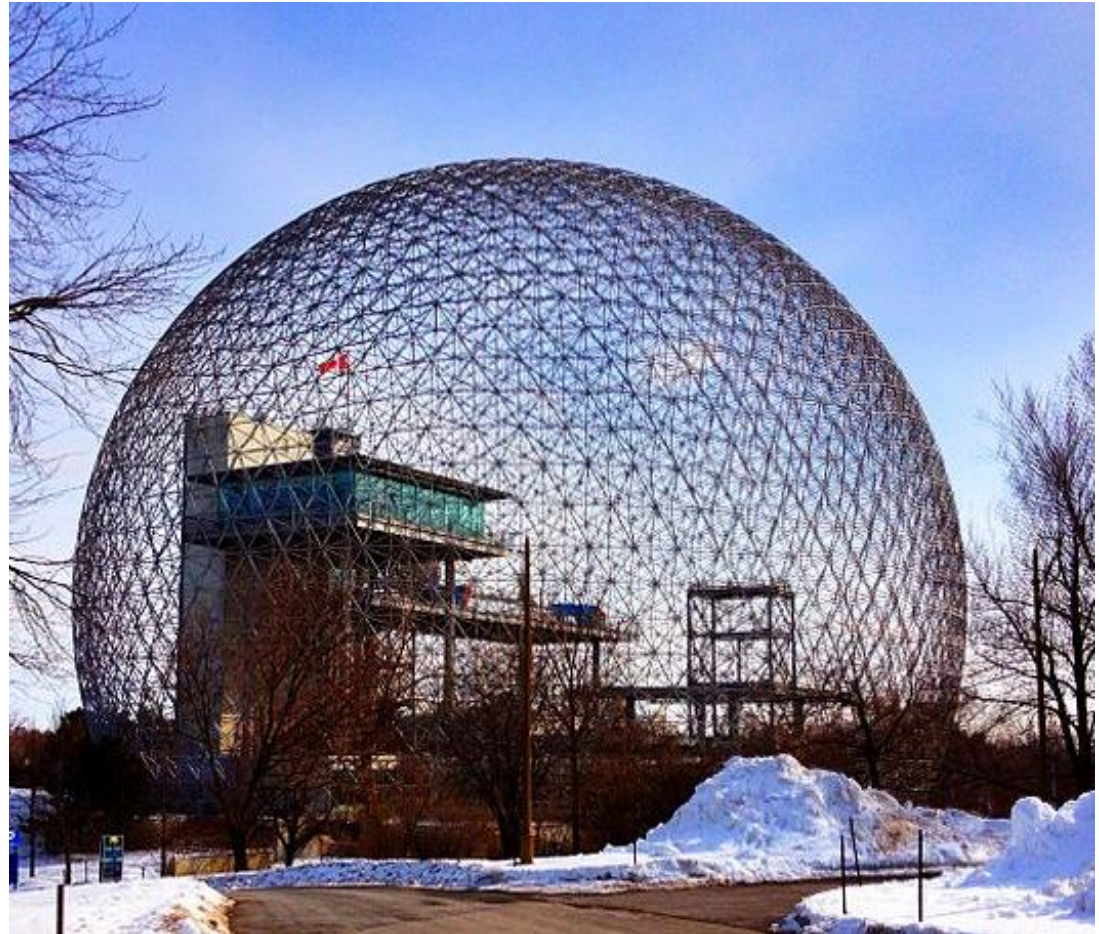


Photo: wikipedia

Hejnalica: 82 m;

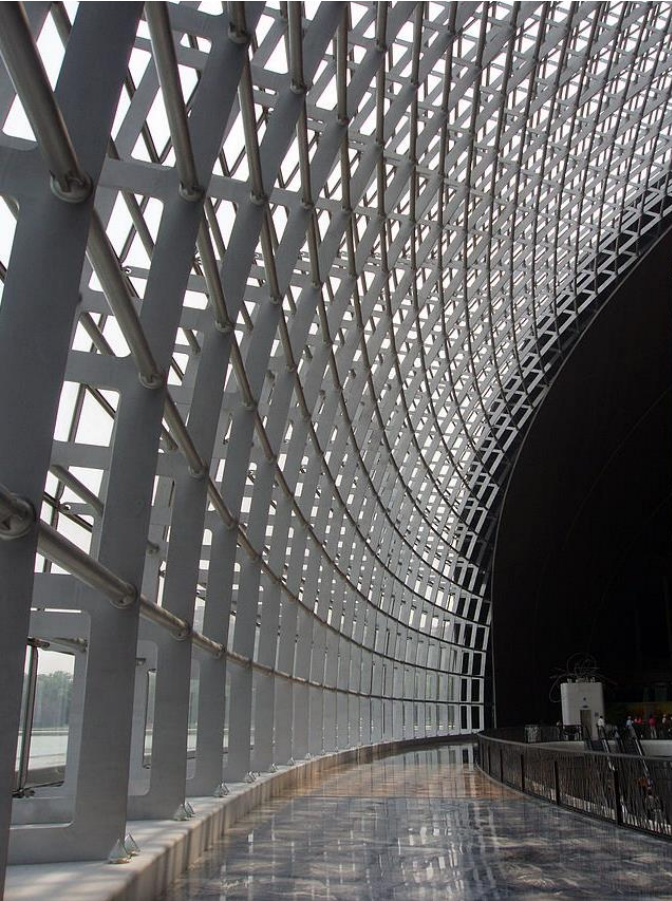


Photo: wikipedia



## National Centre for the Performing Arts, Beijing

length 212 m  
width 144 m  
height 46 m

Eliptical dome

Kraków Main Square: 200x200 m;  
Hejnalica: 82 m;

Photo: wikipedia

## National Sport Stadium, Singapore

diameter 310 m  
height 85 m

Complex of arches,  
retractable roof.

The world's largest dome  
structure

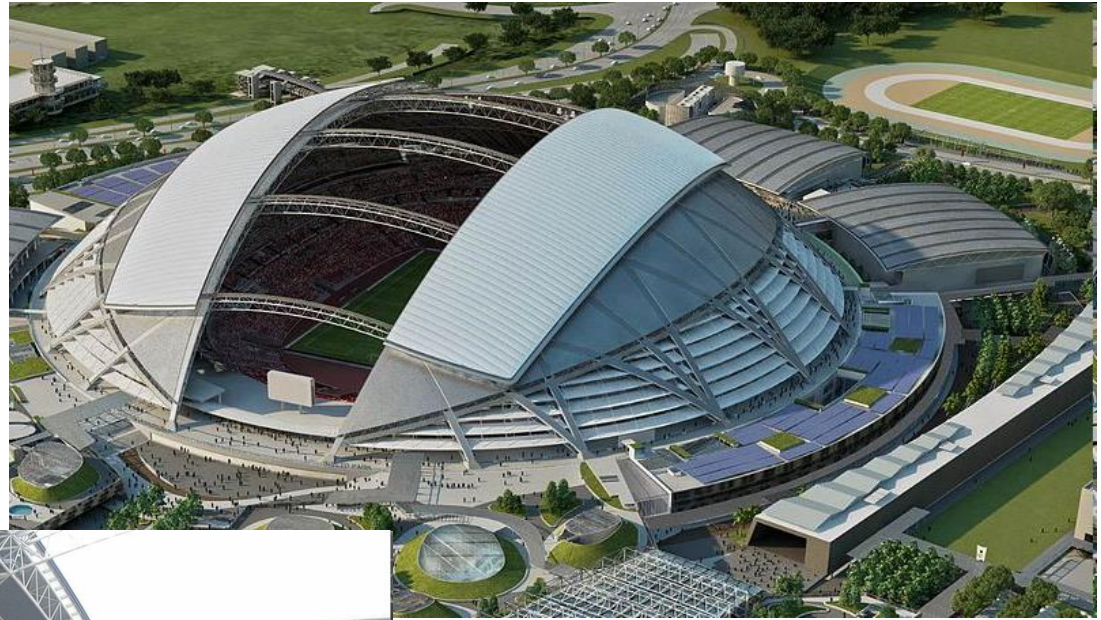


Photo: wikipedia

Kraków Main Square: 200x200 m;  
Hejnalica: 82 m;

Tauron Arena, Kraków

Soprt arena, entertainment center

diameter 166 m

height 27 m

Eliptical dome



Photo: wikipedia

Kraków Main Square: 200x200 m;

# Houston Astrodome

Sport arena

diameter 220 m  
height 63 m

Dome



Kraków Main Square: 200x200 m;  
Hejnalica: 82 m;

Photo: ballparksofbaseball.com

# Ferrari Wolrld, Abu Dhabi

Entertainment center

diameter (total) 775 m  
height 62,5 m



Photo: arhinovosti.ru

Spatial truss



Photo: themeparkreview.com

Distance: Cracow University of  
Technology - Wawel Hill: 2 000 m;

Photo: satimagingcorp.com



Disney Spaceship Earth,  
Orlando

Entertainment center

diameter 50 m

Spherical dome



Photo: wikipedia

Hejnalica: 82 m;

## Tropical Island (Aerium), Dahme-Spreewald

Aquapark, former airship hangar

length 360 m

width 210 m

height 107 m

Complex of arches



Photo: wikipedia



Photo: wikipedia

Kraków Main Square: 200x200 m;  
Hejnalica: 82 m;

Hauptbahnhof,  
Frankfurt

Railway station

large halls:  
length 186 m  
width 50 m  
height 28 m

Complex of arches



Photo: wikipedia

small halls:  
length 186 m  
width 31 m  
height 20 m



Photo: wikipedia

Kraków Main Square: 200x200 m;  
Hejnalica: 82 m;

# Suvarnabhumi Airport, Bangkok

Wing parts  
length 3 213 m  
width 40 m  
height 25 m

Complex of arches and  
suspension roof



Photo: wikipedia

Main part  
length 444 m  
width 111 m  
height 28 m

Spatial truss

Kraków Main Square: 200x200 m;  
Hejnalica: 82 m;  
Distance: Cracow University of  
Technology - Wawel Hill: 2 000 m;



Photo: wikipedia

Photo: wikipedia

## New Safe Confinement shield for Czarnobyl reactor

length 150 m  
width 270 m  
height 105 m

Steel arches and cylindrical shell



Kraków Main Square: 200x200 m;  
Hejnalica: 82 m;



Photo: wikipedia

Lime storage, Yunnan Kungang

Industry hall

length 148 m

width 125 m

height 30 m

Circular shell

Photo: xzlf.en.hisupplier.com

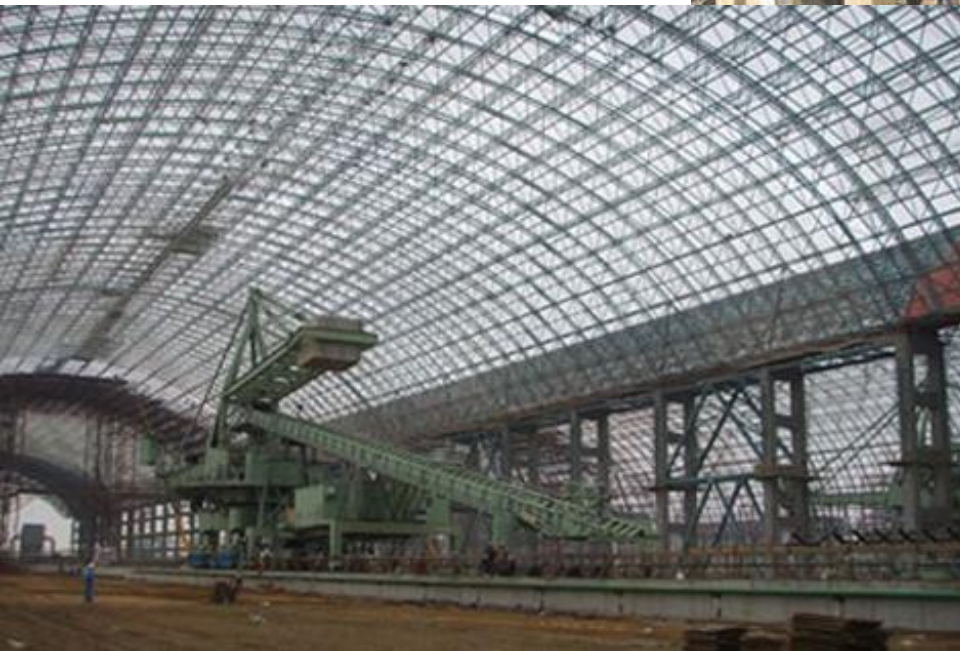


Photo: xzlf.en.hisupplier.com

Kraków Main Square: 200x200 m;

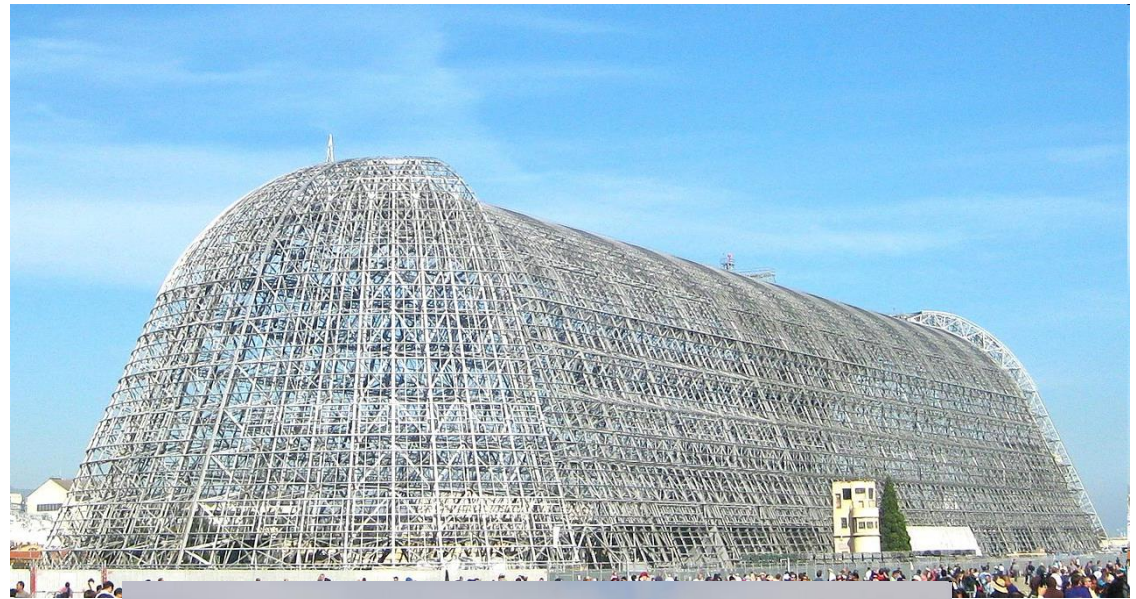
Hejnalica: 82 m;

## Hangar One, Mountain View

Airplane hangar, former  
airship hangar

length 345 m  
width 94 m  
height 60 m

Cylindrical shell



Kraków Main Square: 200x200 m;  
Hejnalica: 82 m;



Photo: wikipedia

Photo: theguardian.com

Airplane hangar, Pekin

length 352 m  
width 110 m  
height 38 m

Spatial truss



Photo: rodaxx.com

Kraków Main Square: 200x200 m;  
Hejnalica: 82 m;

# Gateway Arch, Saint Louis

Landmark

span 192 m  
height 192 m

Steel arch



Photo: wikipedia

Kraków Main Square: 200x200 m;  
Hejnalica: 82 m;

Ring of Life, Fushun

Landmark

diameter 157 m

Steel arch



Photo: dailymail.co.uk

Kraków Main Square: 200x200 m;

Hejnalica: 82 m;

Photo: wikipedia

## Chaotianmen Bridge, Chaotianmen

length 552 m  
height 142 m

Steel arch



Kraków Main Square: 200x200 m;

Hejnalica: 82 m;

## Examination issues

"Light" hall and "heavy" hall, similarities and differences

Technical solutions for long span structures

Leaping instability, short characteristic

Roofing - pokrycie dachu  
Housing - obudowa ścian  
Sandwich panels, wall laminboard - płyty warstwowe  
Cladding panels - panele ściennie  
Self-tapping screw - wkręty samogwintujące  
Purlin - płatew  
Girt - rygiel obudowy  
Castellated beam - belka ażurowa  
Tie-beam - ścią  
Frame - rama  
Tied frame - rama ze ściągiem  
Girder - dźwigar  
Ridge - kalenica  
Eaves - okap  
Drifting - zaspy  
Multi-bay hall - hala wielonawowa  
Multi-span roof - dach wieloprzesłowy (wielopółaciowy)  
Plate girder - blachownica  
Spatial truss / space frame - przekrycie strukturalne

Thank you for attention

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